



A Road Safety Audit

conducted for the

**New Mexico Department of Transportation
(NMDOT)**

in conjunction with the

City of Farmington

for

NM 516 (Main St.) at English Road Intersection

**NMDOT District 5
October 2016**

Conducted by the Road Safety Audit Team



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I. Introduction

I.A Scope and Purpose of the Road Safety Audit (RSA)

The NMDOT received funding under the Federal Highway Safety Improvement Program (HSIP) to conduct a Road Safety Audit (RSA) for existing conditions at the intersection of NM 516 (Main St.) at English Road (approximately Milepost 3.9), located within the City of Farmington in San Juan County, NM. The objective of the study is to offer traffic safety recommendations for smooth traffic flow and reduce the number of crashes at the intersection. The request was submitted by the City of Farmington in February, 2016. The purpose of the RSA is to evaluate the study area, including accident history, for any potential roadway or safety deficiencies.

The scope of the project includes pre-study preparation, data collection, data analysis, stakeholder workshop, and documentation. As part of the data analysis effort, the RSA team reviewed previous studies that are related to this intersection. The RSA study team conducted a workshop to brainstorm the issues and develop countermeasures to address any safety deficiencies identified by the study.

I.B Project Location

The project location is the intersection of NM 516 with English Road (approximately MP 3.9). Information was collected for the approaches in each direction from the intersection. A vicinity map is provided in Figure 1.



Figure 1. Vicinity Map

I.C Items Reviewed

The intersection and approaches were studied to evaluate the conditions for safe traffic flow and to reduce the number of crashes that occur. The data that was collected and reviewed included traffic counts, pedestrian counts, accident data, roadway typical section elements, construction drawings, previous studies, stakeholder input, site observations, and the stakeholder workshop.

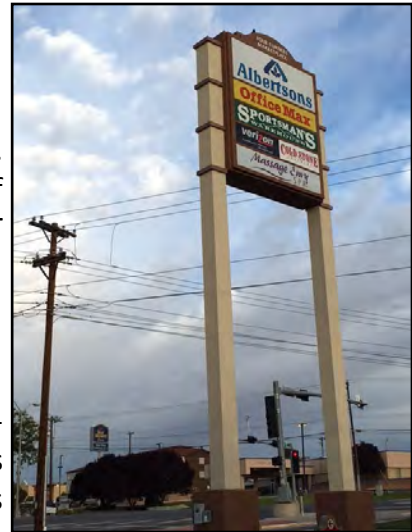
II. Background

II.A Audit Team and Affiliation

The RSA team organized for this study is identified in Table 1. The RSA team consisted of representatives from the City of Farmington, NMDOT General Office—Traffic Section, NMDOT District 5, and Occam Engineers Inc. (OEI).

II.B Road Safety Audit Process

The RSA process involves specific steps that allow and encourage stakeholders to assist with locating potential safety issues and contribute ideas that help solve those issues. This process includes the following steps.



First, a scoping/kick-off meeting was held on Thursday, September 8, 2016 at the City of Farmington offices to discuss the general overview of the RSA and to identify specific issues/locations throughout the corridor. From this meeting, the project scope was discussed and defined. Meeting notes and a list of attendees can be found in the Appendix.

Next, a workshop and site visit was conducted on Tuesday, October 11, 2016 with the RSA workshop team. The site visit was conducted at 8:00 a.m. The workshop was held after the site visit at 9:45 a.m. at the City of Farmington offices. The study background was presented and a discussion of the site visit followed.

The team subsequently entered into a brainstorming session, discussing known operational conditions and observations from the site visit and walk-through. Upon brainstorming the issues, RSA workshop team members listed safety issues at the intersection. After the issues were identified, possible countermeasures were identified for consideration and evaluation or vetting.

After the team developed the list of issues and countermeasures, OEI then consolidated them with the assistance of the workshop participants. The final countermeasures are discussed in greater detail in this report.

Table 1. Road Safety Audit Team	
NMDOT General Office	Afshin Jian, PE State Traffic Engineer
NMDOT District 5	Paul Brasher, PE District Engineer
City of Farmington	Charlie Trask, PE
City of Farmington	David Sypher, PE
City of Farmington	Nica Westerling, PE
City of Farmington	Isaac Blue Eyes, EI
Occam Engineers Inc.	Carlos Ruiz, PE Senior Project Manager
Occam Engineers Inc.	Art Garcia, PE Project Engineer
Thompson Engineering Consultants	David Thompson, PE Project Engineer

Comments from the project team are reflected in the meeting notes from the first meeting held on September 8, 2016 and the Field Review/Workshop held on October 11, 2016, and are included in the Appendix.

II.C Data Collection

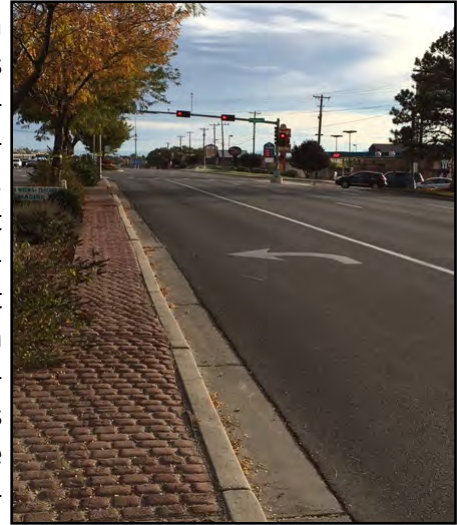
II.C.1. Existing Roadway Conditions

The intersection of NM 516 (Main St.) with English Road is signalized. There is existing corridor street lighting in the median. The medians on NM 516 are raised with concrete curb and are landscaped with bushes and small trees. Crosswalks are located on all four approaches. English Road intersects NM 516 at about a 20 degree skew from perpendicular.

The existing speed limit is 35 mph on NM 516 and 30 mph on English Road.



West Approach. Eastbound NM 516 approaching English Road is a four-lane section with curb and gutter. There is sidewalk that is separated from the roadway by a landscaped buffer. The right lane is a combined through/right-turn lane with no signing or striping to indicate a right turn. There is an exclusive left-turn lane; however, it is short (about 220 feet long) with limited storage due to an additional left-turn lane / median break located about 300 feet west of the intersection that accesses a commercial area on the north side. There is a signalized intersection approximately 1,000 feet west of English Road that provides access to commercial areas north and south of NM 516. There are no entrances to NM 516 from the south in the section between the two signalized intersections.



Westbound NM 516 departing from English Road is a three-lane section with curb and gutter. Sidewalk is adjacent to the curb. There are two entrances in this section.

East Approach. Westbound NM 516 at English Road is a five-lane section with exclusive left-turn and right-turn lanes. There is curb and gutter with sidewalk adjacent to the curb. The westbound to southbound left-turn storage lane is short (about 180 feet). Approximately 550



feet east of the intersection is a median turn lane (combined eastbound to northbound and westbound to southbound) for access into commercial areas. This turn lane impacts the ability to lengthen the left-turn storage at English Road. The westbound to northbound right-turn storage lane is approximately 400 feet long, with a raised turn island and yield control for merging in the northbound lanes with English Road traffic.

The closest, adjacent signalized intersection from this approach is Pinon Hills Blvd, which is approximately 0.6 miles east of English Road. There are six entrances on the north side of NM 516 between these two intersections.

Eastbound NM 516 departing English Road is a four-lane section for about 500 feet from the intersection. The right lane is an exclusive acceleration lane that doubles as a right-turn lane for access to the commercial area south of NM 516. There is a right turn only sign at the east end of this lane; however, there is no signing at the beginning of this lane to indicate through traffic must merge left. East of this location, NM 516 is a three-lane section. There is curb and

gutter with sidewalk adjacent to the curb. There are two city streets and at least nine commercial entrances on the south side of NM 516 between English Road and Pinon Hills Blvd.

South Approach. English Road is a three-lane section south of NM 516. However, in the immediate vicinity of the intersection, English Road is a four-lane section which includes exclusive left-turn and right-turn lanes with limited storage capacity (140 feet or less). There is no turn lane indication (sign) in advance of the intersection. The northbound to eastbound right-turn lane has a raised turn island and yield control since there is an exclusive merge lane on NM 516.



Southbound English Road south of NM 516 is one lane. There is curb and gutter on both sides of English Road, with sidewalk adjacent to the curbs. There are bike lanes on both sides of English Road that begin several hundred feet south of the intersection.

North Approach. Immediately north of NM 516, southbound English Road is a three-lane section with short left-turn (about 160 feet) and right-turn (100 feet) lanes in addition to the through lane. There is curb and gutter on both sides of English Road, with sidewalk adjacent to the curbs. There is no turn lane indication (sign) in advance of the intersection.

Northbound English Road north of NM 516 is two lanes. There are bike lanes on both sides of English Road that begin several hundred feet north of the intersection.



Median Break West of Intersection. Approximately 300 ft west of the English Road intersection there is an existing median break with a left turn storage lane. This median opening is apparently to allow eastbound (EB) vehicles to turn left into a commercial entrance without having to go through the English Road intersection. The median break seems excessively long, and would appear to allow several potentially unsafe turning movements in addition to the EB left-turns. Specifically, vehicles departing from the entrance on the north side could potentially

make left turns to the EB lanes, and there is nothing to prevent U-turns from occurring at the median opening. In addition, the commercial entrance on the north appears to be confusing to motorists, based on the numerous curb hits (tire marks and broken curbs) evident at the site.



II.C.2. Bicycles and Pedestrians

There was little to no bicycle or pedestrian activity observed during the site visits to the intersection. This was confirmed by the 38-hour counts conducted the weekend of September 30—October 1, 2016: only 3 bicycles on the road and 6 in the crosswalks were noted. There were a total of 115 pedestrians during that same time. The highest number of pedestrians during any one hour was 10. Copies of the counts are located in the Appendix.

II.C.3. Accident Data Collection and Analysis

NMDOT Accident Data. Accident data was obtained from the NMDOT Planning and Traffic Safety Division / Crash Records Bureau for the 5-year period from 2010 through 2014. Due to the limited time available for obtaining and analyzing accident data, the RSA study team did not request accident reports. The accident data is summarized in Tables 2 through 4.

There was a total of 73 crashes in the 5-year study period for the intersection and approaches. Of these, there were 30 injury crashes (41%). There were no fatal crashes. The remaining 43 crashes (39%) were property damage only (PDO). Tables 2 through 4 provide additional information on the accidents reported.

Nearly all of the crashes were caused by some form of driver error. Most of these were typical of intersections: 21 failure to yield, 11 following too closely, 9 driver inattention, 7 disregard traffic signal, 4 improper lane change.

The accident data includes classification by type. Using this system, the most common type was left-turn: 24 accidents (33%). Vehicles going straight, including sideswipes, accounted for 21 accidents (29%). There were 12 rear end (16%) and 5 right-angle (7%) crashes.

Four crashes (5.5%) were alcohol-related. Only two accidents might have had weather (rain and snow) as potential factors.

There were five pedestrian, one bicycle, and three motorcycle accidents – all relatively low numbers.

Tables 2—4. Accident Data Summary

TABLE 2—OVERALL SUMMARY

Year	Number of Crashes	Fatal Crashes	Injury Crashes	Property Damage Only	Dark-not Lighted	Pedestrian Involved	Bicyclist Involved
2010	14	0	5	9	1	1	0
2011	14	0	4	10	0	0	1
2012	17	0	7	10	0	0	0
2013	15	0	9	6	0	4	0
2014	13	0	5	8	0	0	0
Total	73	0	30	43	1	5	1

TABLE 3—HIGHEST CONTRIBUTING FACTORS

Year	Number of Crashes	Failed to Yield	Driver Inattention	Disregarded Traffic Signal	Improper Lane Change	Following Too Closely	Alcohol or Drugs Involved	Excessive Speed	Other
2010	14	6	4	1	1	1	0	1	0
2011	14	4	0	2	2	4	1	0	1
2012	17	3	2	3	0	3	2	2	2
2013	15	5	2	0	0	2	0	0	6
2014	13	3	1	1	1	1	1	0	5
Total	73	21	9	7	4	11	4	3	14

TABLE 4—CRASH ANALYSIS

Year	Number of Crashes	Right Angle	Left Turn	Both Going Straight Same Direction	Rear End	Sideswipe	Other
2010	14	1	5	3	1	2	2
2011	14	0	6	3	2	2	1
2012	17	1	2	5	5	1	3
2013	15	2	6	1	3	0	3
2014	13	1	5	4	1	0	2
Total	73	5	24	16	12	5	11

Fourteen accidents (19%) were at night. There is existing street lighting; therefore, only one accident was reported as being Dark-Not Lighted.

There was one animal accident involving a deer.

City of Farmington Accident Data. This data came from two sources: the City of Farmington Police Department (PD) which included 13 accidents from February 2015 through August 2016; and from the City of Farmington Traffic Engineering Department, which included 23 accidents from November 2014 through February 2016. The accident data is included in the appendix.



The PD accident data included 5 accidents where no reports were taken, so information on those accidents is not available. Of the remaining 8 accidents, 6 involved rear-end crashes, and the rest were a combination of running a red light (Failure to Yield) and right-angle. These types of accidents are common at signalized intersections. Two involved injuries; the remaining 6 were PDO.

The Traffic Department data set include 5 accidents already included with the PD, and 2 accidents that were already within the NMDOT accident listing. The remaining 16 accidents were in addition to any other data set; however, there was no information provided other than date and time, so no analysis was performed on this data set.

II.C.4. Existing Traffic Data

The intersection of NM 516 with English Road, and adjacent streets and entrances, serve a number of businesses including a few large traffic generators such as Animas Valley Mall, Target, Albertson's, and other businesses including sporting goods, restaurants, banks, hotels, and an urgent care center. There are also residential areas located a few blocks north and south of the intersection. These land uses generate significant



traffic at the intersection.

The average annual daily traffic (AADT) for this intersection was obtained from the NMDOT website. The AADT for 2015 was estimated by NMDOT, and published in June 2016, based on actual counts taken in 2011. The 2015 AADT was 24,351. Heavy vehicles were reported at 11%. The AADT report is included in the appendix.



II.C.5. Turning Movement Traffic Counts

Turning movement counts were obtained in the field from 6:00 a.m. on Friday, September 30, 2016 through 8:00 p.m. on Saturday, October 1, 2016 for a total of 38 hours. Traffic flow data was collected in fifteen minute intervals to determine the peak hours. Classifications were conducted to determine the flow of vehicles, bicycles, and pedestrians. The Peak Hours were then determined.

Figures 2 through 5 contain the highest Peak Hour Turning Movements which occur mid-day on both Friday and Saturday. The turning movement counts did not indicate any A.M. or P.M. peak hours. Complete turning movement counts are provided in the Appendix.

II.D Other Documents Reviewed

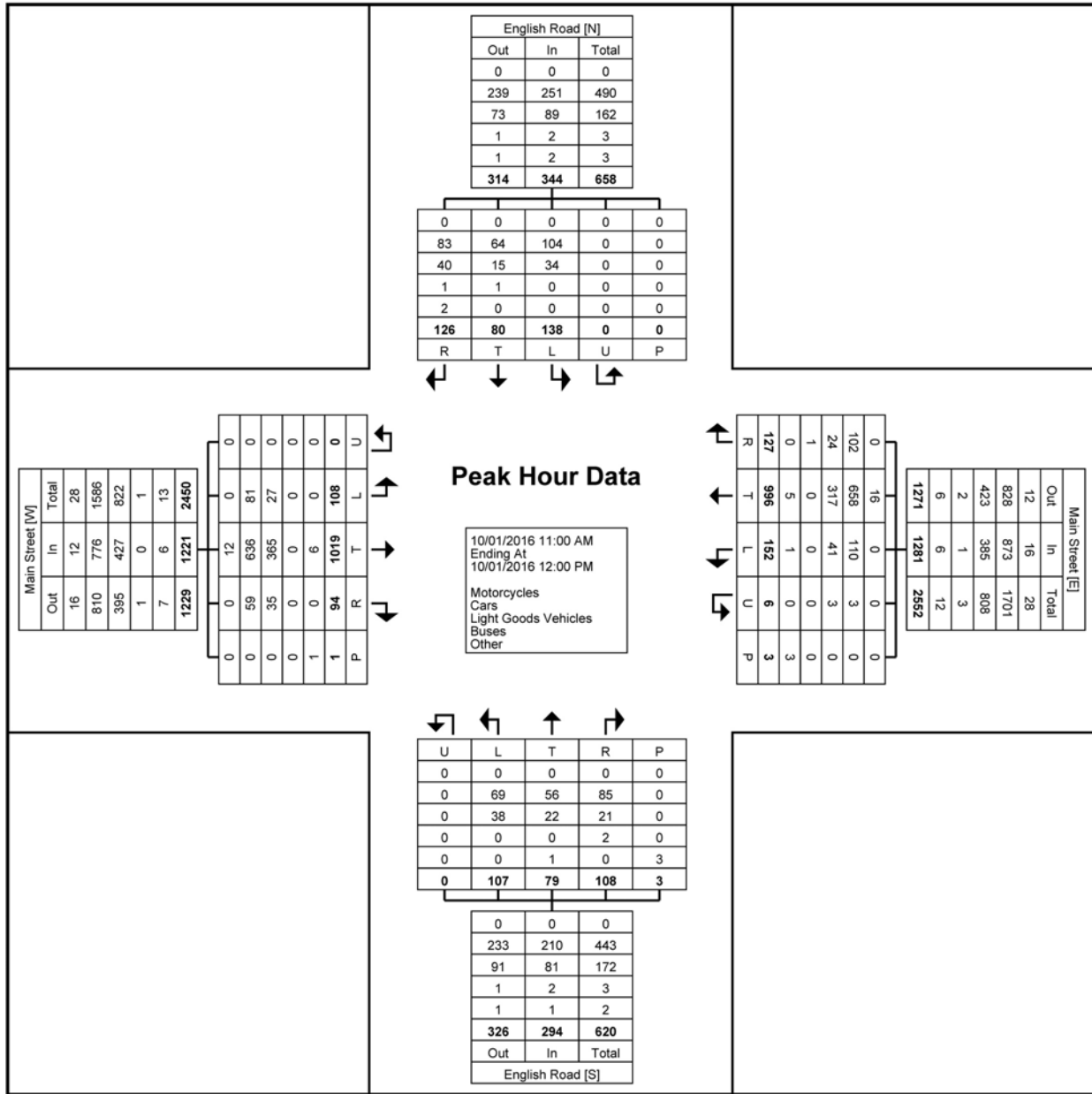
As part of the RSA team’s analysis of the study area, **OEI** obtained and reviewed several recent studies related to the project. The studies are listed in the References Section at the end of this report. The studies reviewed included a traffic impact analysis for a nearby commercial entrance. **OEI** requested previous traffic studies from NMDOT District 5 and the City of Farmington but were informed that none were available at this intersection.

II.E Site Visit General Observations

The study team conducted a site visit on the morning of the RSA workshop on October 11, 2016. The observations from the RSA workshop are provided in the meeting notes from the field review workshop. These can be found in the Appendix.

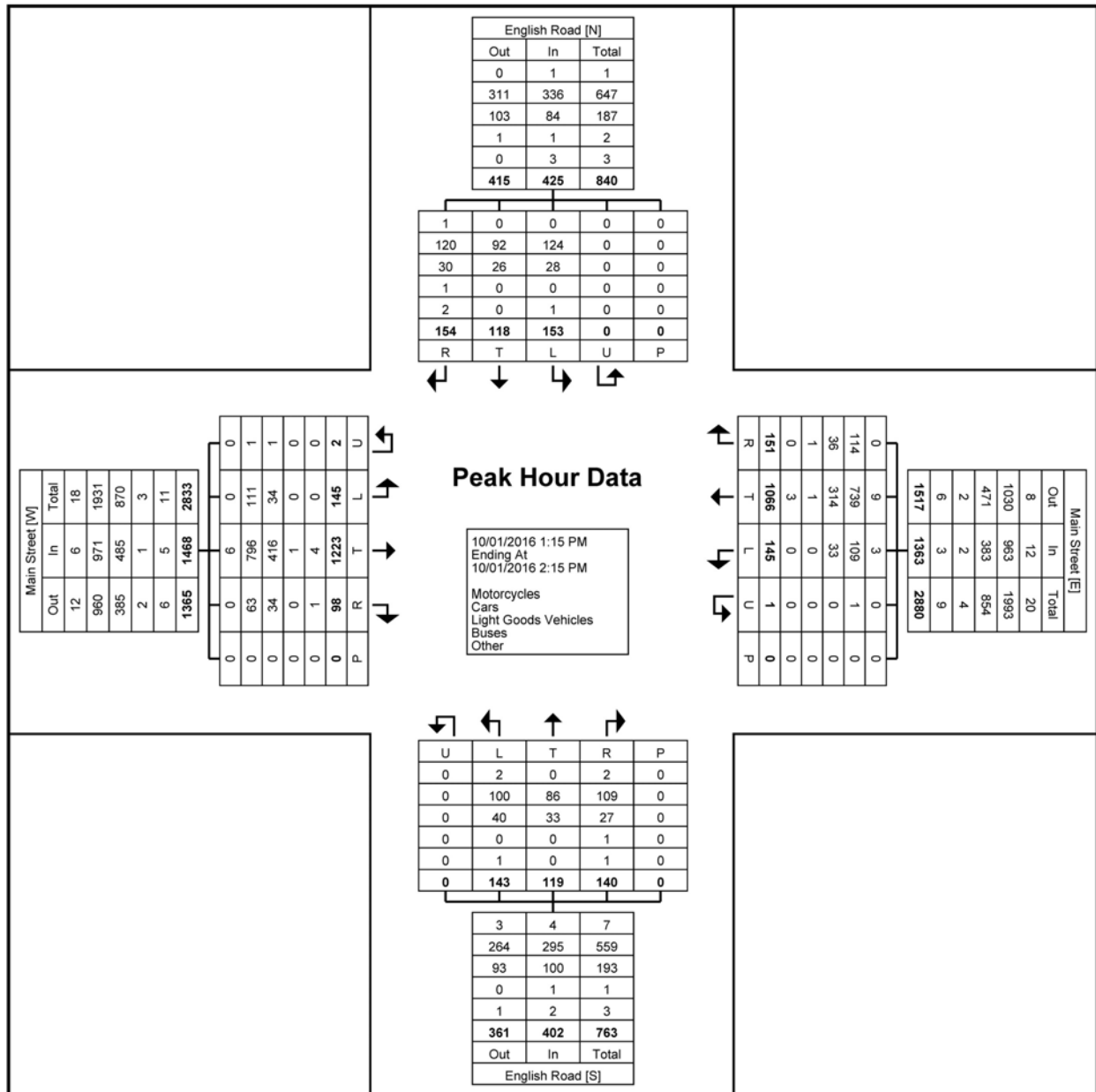
Aerial photographs from Google and pictures taken of the project area by **OEI** are provided in the Appendix.

Figure 4. Saturday Peak Hour Turning Movements



Turning Movement Peak Hour Data Plot (11:00 AM)

Figure 5. Saturday Peak Hour Turning Movements



Turning Movement Peak Hour Data Plot (1:15 PM)

III. Findings and Countermeasures

Below is a list of the Issues and the countermeasures that were identified during the workshop held on October 11, 2016.

III.A Issues Identified

The following issues were identified by the RSA study team during the workshop:

1. Capacity / operations
2. Medians
3. Driveways
4. Pedestrian / ADA
5. Geometric / median nose
6. Vertical alignment in the north-south direction
7. Skewed alignment
8. Signing / striping / painting on noses at medians and driveway entrance
9. Lighting

These issues above were consolidated into five main categories:

- 1. Medians, driveways, geometrics, vertical alignment**
- 2. Striping and Signage**
- 3. Pedestrian / ADA**
- 4. Capacity / operations**
- 5. Lighting**

III.B Countermeasures

Countermeasures were identified during the workshop for each issue “category” defined above, and are listed under the applicable category below:

- 1. Medians, driveways, geometrics, vertical alignment**
 - Raise grades on English Road south of the intersection
 - Modify medians:
 - extend median nose at east intersection approach
 - shorten nose at west intersection approach and convert to double left turn
 - extend both noses at median break west of intersection
 - Close median break west of intersection
 - Reconfigure commercial driveway
- 2. Striping and Signage**
 - Add signs to supplement painted markings. The following are recommended signs:

- One sign in advance of northbound approach on English to indicate turn/through lanes
 - One sign in advance of southbound (SB) approach on English to indicate turn/through lanes
 - Two No U-Turn signs (both directions) at median break west of intersection
 - One No Left-Turn sign at driveway exit in NW quadrant of intersection
 - One advance sign for EB traffic to indicate median break is for access to the commercial area (blue/white sign)
 - One advance sign for EB traffic to indicate left turn for English is ahead.
 - Two signs for EB (one at median break, one at intersection) indicating Left Turn Only lane
 - One sign at beginning of EB acceleration lane (SE quadrant) to indicate Lane Ends / Merge Left
 - Add striping including turn arrows, crosswalks
 - Paint median and driveway curb noses with yellow reflective paint
- 3. Pedestrian / ADA**
- Pedestrian crossing timing
 - Meet ADA: ramps / width / detectable warnings
 - Raise grades on English Road south of intersection
- 4. Capacity / operations**
- Perform capacity analysis
 - Based on analysis, improve operations and queuing
- 5. Lighting**
- Add intersection lighting in addition to median lighting

III.C Discussion of Countermeasures Eliminated

Several of the countermeasures are considered functions of routine maintenance, and are therefore assumed to be budgeted and scheduled within the NMDOT District 5 budget for maintenance and operations. While this group of countermeasures are included as recommendations in this report (see Conclusions section), since they will not ultimately become a safety project they have been eliminated from further evaluation of countermeasures, including determination of project costs, for purposes of this RSA. These include the following countermeasures:

- Add striping including turn arrows, crosswalks (this work has been scheduled)
- Paint median and driveway curb noses with yellow reflective paint
- Pedestrian crossing timing
- Based on analysis, improve operations and queuing

A capacity analysis was performed as part of this RSA using peak hour traffic collected for this study. The traffic signals at this intersection use an adaptive timing scenario, which continually adjusts signal timing. Therefore, the analysis performed for this RSA only illustrates conditions based on the background timing settings, i.e., the starting point for which the system will make adjustments. Based on these assumptions, the analysis determined that for the peak hour (12:30-1:30 pm), the EB, NB and SB approaches are currently operating at a Level of Service (LOS) "D" which is acceptable. The westbound (WB) approach is operating at a LOS "E." The critical movement for this approach is the WB to SB left-turn which is at a LOS "F." The intersection overall is operating at a LOS "D." Signal timing changes for the intersection (assuming current geometric configuration) could potentially improve the LOS "F" for this movement. Output from the capacity analysis is included in the Appendix.

III.D Final Countermeasures

The remaining countermeasures were consolidated into the following five countermeasures:

III.D.1. Raise Grades on English Road South of the Intersection

The existing grade of the south intersection approach falls as English Road goes south. This countermeasure would involve reconstructing the vertical alignment of the south approach to make it more level with NM 516. The redesign would improve the grade of the crosswalks on NM 516 to bring them into compliance with ADA. However, in order to "daylight" English Road to the south would require a long transition, thereby increasing the construction cost. ADA improvements on the south side of NM 516 are included in this countermeasure.

III.D.2. Modify Medians and Reconfigure Commercial Driveway

This includes the extension of three median noses and the reduction of one nose, as described above. The EB to NB movement would be changed to a dual left-turn. The commercial driveway on the north side of NM 516 would be reconfigured to restrict SB traffic from turning left onto EB NM 516. This item includes all required signing, as recommended above. Closing the median break is not included but could be considered as an additional countermeasure. A sketch of the proposed improvements is provided as Figure 6 on the following page.

III.D.3 Add Signs to Supplement Painted Markings

This countermeasure would add approximately 10 signs. The locations and types of signs are described above.

III.D.4 Meet ADA: Ramps / Width / Detectable Warnings

The NE quadrant of the intersection appears to meet ADA requirements, so this countermeasure would involve the other three quadrants. The ADA ramps at each corner would be modi-

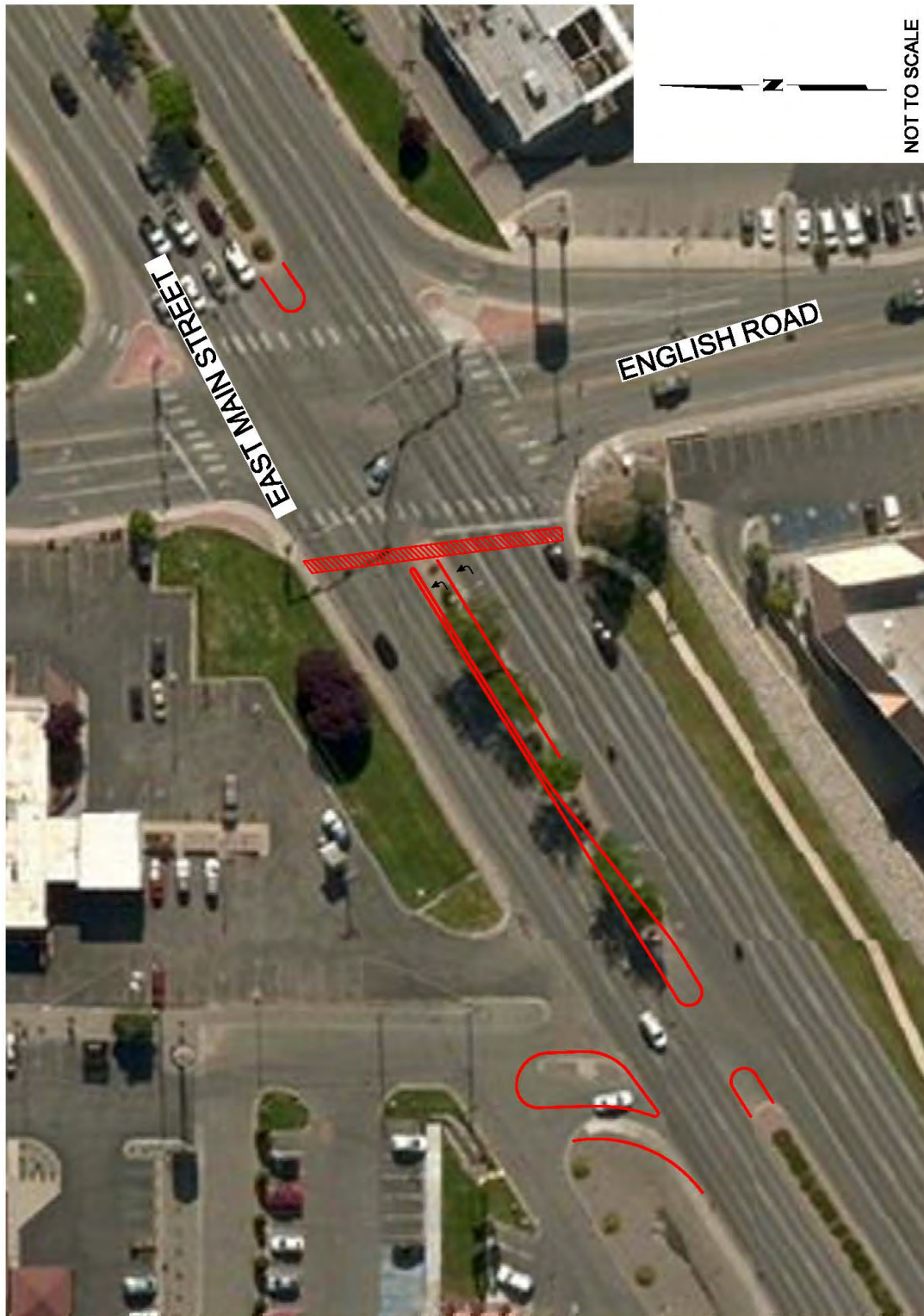


Figure 6. Proposed Median Improvements

fied including the addition of detectable warning surfaces, widening sidewalks, relocating the ramps as needed to meet slope requirements, and installation of signal push buttons for pedestrians at the NW and SW corners.

III.D.5 Add Intersection Lighting

Approximately four light standards would be added for this countermeasure.

III.E Conclusions

The countermeasures listed in Table 5 are recommended for programming into the process for implementation. Estimated costs are provided for each countermeasure for budget level plan-

<u>Countermeasure</u>	<u>Planning Level Cost Estimate</u>
<p>1. Raise Grades on English Road South of the Intersection Includes reconstruction of approximately 400 feet of English Road south of NM 516. Does not include right-of-way or easement acquisition or utility adjustments.</p>	\$420,000 to \$550,000
<p>2. Modify Medians and Reconfigure Commercial Driveway Includes the following improvements:</p> <ul style="list-style-type: none"> • extend median nose at east intersection approach • Add EB to NB dual left-turn • shorten nose at west intersection approach and convert to double left turn • extend both noses at median break west of intersection 	\$108,200 to \$143,400
<p>3. Add Signs to Supplement Pavement Markings Approximately 10 new signs.</p>	\$10,800 to \$14,300
<p>4. Meet ADA: Ramps / Width / Detectable Warnings The ramps at the NW, SW, and SE corners would be updated to meet ADA including detectable warning surfaces, wider sidewalks, relocating the ramps as needed to meet slope requirements, and installation of signal push buttons for pedestrians (NW and SW corners).</p>	\$69,300 to \$91,900
<p>5. Add Intersection Lighting Four intersection light standards with conduit, wiring, and other required items.</p>	\$64,400 to \$85,300

ning. ***The costs include amounts for engineering design, construction administration, New Mexico gross receipts taxes, and contingencies.***

In addition to the countermeasures in Table 5, the following countermeasures are recommended for implementation by the appropriate agencies including the NMDOT:

- Add striping including turn arrows, crosswalks (this work has been scheduled)
- Paint median and driveway curb noses with yellow reflective paint
- Pedestrian crossing timing
- Revise signal timing to improve LOS for WB left-turn movement

OEI would like to thank the City of Farmington for providing meeting space.

IV. References

Highway Safety Manual, 1st Edition, American Association of State Highway and Transportation Officials, 2010

Manual on Uniform Traffic Control Devices (MUTCD) for Streets and Highways, 2009 Edition. American Association of State Highway and Transportation Officials

New Mexico Department of Transportation State Access Management Manual, 2001 Edition

New Mexico Department of Transportation Standard Drawings, <http://www.dot.state.nm.us/en/Standards.html>

New Mexico Highway Safety Improvement Program (HSIP) Road Safety Audit (RSA) Manual, New Mexico Department of Transportation, March 24, 2014

New Mexico Traffic Crash and DWI Statistics, University of New Mexico Geospatial and Population Studies, Traffic Research Unit, <http://www.dgr.unm.edu/>.

FHWA Road Safety Audit Guidelines, Publication No. FHWA-SA-06-06, U.S. Department of Transportation, Federal Highway Administration, 2006, Ward, Louisa, Project Manager

New Mexico Department of Transportation Statewide Transportation Improvement Plan (STIP) Amendment 3, U.S. Department of Transportation, Federal Highway Administration, July 16, 2016

U.S. New Mexico Federal Credit Union (NM S.R. 516 / Pinon Hills Blvd—Farmington, NM), Traffic Impact Study, Terry O. Brown, PE, Albuquerque, NM, July 28, 2014

V. Appendix

A. Meeting Notes & Sign in Sheets

B. 2010-2014 Accident Data

C. Aerial Photographs

D. Photographs

E. Traffic Data

F. Traffic Data Analysis

Appendix A—Meeting Notes

NMDOT Road Safety Audit (RSA)

NM 516 (MAIN STREET)/ENGLISH ROAD INTERSECTION

NMDOT CN 910011

Kick-Off Meeting, Thursday, September 8, 2016, 11:00 a.m.

A kick-off meeting was held for the Road Safety Audit (RSA) for NM 516 (Main Street)/English Road Intersection on Thursday, September 8, 2016 at 11:00 a.m. at the City of Farmington Office. The following individuals were in attendance:

Afshin Jian	NMDOT General Office
David Sypher	City of Farmington
Nica Westerling	City of Farmington
Charlie Trask	City of Farmington
Isaac Blue Eyes	City of Farmington
Carlos Ruiz	OCCAM
Dave Thompson	TEC

Everyone at the meeting introduced themselves. Carlos Ruiz discussed the agenda for the kick-off meeting. Mr. Ruiz explained what an RSA is and what the result will be when it is completed. A general overview of the project limits was discussed. The following are specific items that were brought up during the meeting:

- The HSIP Application was initiated so that the City could build double left turn lanes from eastbound Main Street to Northbound English Road.
- The intersection cross slope is greater than 5% so it does not meet ADA standards. South side of Main Street from the “Porkchop” to the crosswalk is not viable to use for handicap pedestrians.
- The stacking length of the existing turn lane is not long enough.
- Information that we will be obtaining include: Crash Data, As-builts, and Traffic Counts.
- Any traffic counts should be taken during the first weekend of the month as that is the busiest time.

Appendix A—Meeting Notes

- Charlie has traffic counts from 2014 that he will email to us.
- Charlie has more recent crash data for 2015 and the first 3 months of 2016 that he will email to us.
- Carlos suggested that we take new turning movements counts. It was agreed that these turning movements will be taken from 6 a.m. Friday, September 30th to 8 p.m. Saturday, October 1st.
- We will count bicycles, pedestrians, and animals.
- Charlie requested that a capacity analysis be completed and that we examine queuing in the turn bays.
- Nica and Charlie said that District 5 has several recent traffic studies in the area that were completed for new developments.
- The existing break in the median of the eastbound turn lane at the Main/English intersection provides access to commercial properties on the north side of Main. This could contribute to the queuing length in the turn lane.
- Other Stakeholders to invite to the workshop include: 1) NMDOT District 5; 2) Fire Station Representative; 3) Police Substation Representative; 4) City of Farmington Planning; and 5) MPO Representative.
- The workshop date and place is scheduled for Tuesday, October 11th at 8:00 a.m. at the City of Farmington.

Appendix A—Meeting Notes

**NMDOT Road Safety Audit (RSA)
NM 516 at English Road - MP 3.9
NMDOT CN 910011
Workshop Meeting and Site Visit
Tuesday, October 11, 2016, 8:00 a.m.**

A workshop meeting and site visit was held for the Road Safety Audit (RSA) for NM 516 at the English Road intersection (MP 3.9) on Tuesday, October 11, 2016 from 8:00 a.m. – 11:30 a.m. at the City offices in Farmington, NM. The attached sign-in sheet provides a record of the individuals that were in attendance.

The study team met at the site and discussed potential issues at the intersection and approaches. The meeting followed at the City of Farmington offices. Carlos Ruiz (Occam) explained the purpose of an RSA and what the result will be when it is completed. He mentioned that the team will look at efficiency only to the extent that it will affect safety.

1. The following potential safety issues were raised:
 - a. Capacity / operations (TBD if this is an issue based on LOS analysis)
 - b. Medians – 4 issues:
 - c. Driveways
 - d. Pedestrian / ADA
 - e. Geometric / median nose
 - f. Vertical N-S
 - g. Skewed alignment
 - h. Signing / striping / painting on noses at medians and driveway entrance
 - i. Lighting (there is corridor lighting but no intersection lighting)
2. The issues identified above were consolidated into groups as defined below, and countermeasures were suggested for each group:
 - a. Group 1 – medians, driveways, geometrics, vertical alignment
 - i. Raise grades on South English
 - ii. Modify medians:
 1. extend median nose at east intersection approach
 2. shorten nose at west intersection approach and convert to double left turn
 3. extend both noses at median break west of intersection
 - iii. Close median break west of intersection
 - iv. Reconfigure commercial (urgent care, etc.) driveway
 - b. Group 2 – signing, striping
 - i. Add signs to supplement painted markings. The list below are examples:
 1. One sign in advance of NB approach on English to indicate turn/thru lanes
 2. One sign in advance of SB approach on English to indicate turn/thru lanes
 3. Two No U-Turn signs (both directions) at median break west of intersection

Appendix A—Meeting Notes

4. One No Left-Turn sign at driveway exit in NW quadrant of intersection
 5. One advance sign for EB traffic to indicate median break is for access to the commercial area (blue/white sign)
 6. One advance sign for EB traffic to indicate left turn for English is ahead.
 7. Two signs for EB (one at median break, one at intersection) indicating Left Turn Only lane
 8. One sign at beginning of EB acceleration lane (SE quadrant) to indicate Lane Ends / Merge Left
- ii. Add striping including turn arrows, crosswalks
 - iii. Paint median and driveway curb noses with yellow reflective paint
- c. Group 3 – Pedestrian / ADA
 - i. Pedestrian crossing timing
 - ii. Meet ADA: ramps / width / detectable warnings
 - iii. Raise grades on South English
 - d. Group 4 – Capacity / operations
 - i. Perform capacity analysis
 - ii. Based on analysis improve operations and queuing
 - e. Group 5 – lighting
 - i. Add intersection lighting in addition to median lighting

As part of the site visit and subsequent determination of issues and countermeasures, the following items were also discussed during the meeting:

- The missing pavement striping has already been scheduled.
- There are few bikes and only one bicycle related accident. There are no bike lanes on NM 516. The bike lanes on English begin / end several hundred feet away from the intersection. The traffic counts indicated very few bicycles. Bicycles do not appear to be an issue.
- There were 73 accidents at the intersection from 2010-2014. The accidents were spread more or less uniformly over that five-year period. Most accidents were drive error.
- There is corridor lighting in the median on NM 516, which may not be sufficient for the intersection. There are overhead power lines that might restrict the type of lighting that could be added.
- The ramp island in the NE quadrant appears to be ADA compliant; the other three corners do not appear to meet ADA. A potential countermeasure for the SW corner might be to move the crosswalk landing approximately 10-15 ft west, where there is a more gradual grade.

Carlos Ruiz stated that the issues and countermeasures would be included in a draft report that would be e-mailed to all persons in attendance at the meeting. We would need comments back within a week.

Appendix B—Farmington Traffic Engineering Department Accident Data

Caseid	Date	Time
2014-00068955	11/25/2014	4:23 PM
2014-00069788	11/29/2014	6:41 PM
2014-00073334	12/16/2014	5:17 PM
2014-00073920	12/19/2014	2:26 PM
2015-00010175	2/17/2015	4:45 PM
2015-00013830	3/6/2015	4:40 PM
2015-00014214	3/8/2015	4:18 PM
2015-00014671	3/10/2015	6:39 PM
2015-00014671	3/10/2015	6:39 PM
2015-00016241	3/17/2015	6:05 PM
2015-00026258	5/2/2015	7:33 PM
2015-00029770	5/19/2015	4:50 PM
2015-00031668	5/28/2015	8:51 AM
2015-00047959	8/5/2015	4:31 PM
2015-00060073	9/25/2015	12:53 PM
2015-00060603	9/27/2015	4:19 PM
2015-00067927	10/30/2015	7:05 PM
2015-00070715	11/13/2015	2:24 PM
2015-00072195	11/20/2015	2:54 PM
2015-00076310	12/10/2015	9:25 AM
2015-00078013	12/18/2015	2:21 PM
2016-00007562	2/6/2016	1:13 PM
2016-00010964	2/22/2016	4:08 PM

Appendix B—Farmington Police Department Accident Data

Incident Information

CALL DATE	INCIDENT NUMBER	STREET ADDRESS	CALL TIME	INCIDENT TYPE	NATURE OF CALL	CROSS STREET ADDRESS	DAY OF WEEK	Accident details
02/17/2015	20150001017	„ENGLISH,RD,	16:45:5	Traffic	APV/STILL IN ROADWAY	E,MAIN,ST,	Tuesday	Pulling out of Mall heading northbound on English front of V2 hit driver's side of V1 (Side swiped?)
03/08/2015	20150001421	,E,MAIN,ST,,	16:18:4	Traffic Accident w/Injury		,ENGLISH,RD	Sunday	V2 ran red light while on Main and crashed into passenger front of V1 while turning Southbound on English and V3 rear ended V1.
03/17/2015	20150001624	„ENGLISH,RD,	18:05:4	Traffic Accident No Injury	IN THE ROAD/NO INJURIES/	E,MAIN,ST,	Tuesday	Rear ended at light of English and Main
05/02/2015	20150002625	,E,MAIN,ST,,	19:33:5	Traffic Accident No Injury	RP REQ TO REPORT HIT AND RUN	,ENGLISH,RD	Saturday	V1 stopped at light on English and was rear ended V2 (hit and run)
05/07/2015	20150002714	,E,MAIN,ST,,	7:01:19	Traffic Accident No Injury	RP RAN RED LIGHT	,ENGLISH,RD	Thursday	Side swiped V2 ran red light
06/16/2015	20150003590	,E,MAIN,ST,,	10:42:5	Traffic Accident No Injury	OCCURRED 15 MINS AGO/ NO LONGER THERE	,ENGLISH,RD	Tuesday	No report taken. The incident was called into dispatch but nobody there when officers arrived
12/10/2015	20150007631	„ENGLISH,RD,	9:25:05	Traffic Accident No Injury	NO INJ/RP WAS REAR ENDED	E,MAIN,ST,	Thursday	Rear ended at light of English and Main

Appendix B—Farmington Police Department Accident Data

04/15/201	20160002198	5	13:42:2	5	13:42:2	English, RD,	English, RD,	5	5	13:42:2	Traffic Accident	APV/ NO CHEM/ NO HAZ	E, MAIN, ST,	Friday	Rear ended at light of English and Main V3 hit V2 and V2 hit V1 (V1 was stopped at light and V2, V3 were following to close)
05/05/201	20160002644	1	11:53:3	9	11:53:3	English, RD,	English, RD,	9	9	11:53:3	Traffic Accident	APV/WHTIE FORD 4DR - LIFTED - 600TFD	E, MAIN, ST,	Thursday	No report found so either happened on private property or damage was less than 500.00 dollars so no report would be taken
06/01/201	20160003268	4	15:22:2	4	15:22:2	English, RD,	English, RD,	4	4	15:22:2	Traffic Accident	NO INJ VEH ARE NOT IN THE ROADWAY	E, MAIN, ST,	Wednesday	No report found so either happened on private property or damage was less than 500.00 dollars so no report would be taken
Date - Call Close Date and Time	Time - Call Close Date and Time	Call Current Address	Call Current Address Cross Street Name	Call Current Address Cross Street Name	Call Current Address Cross Street Name	Call Type	Call Type	Call Type	Call Type	Call Type	Call Type	Call Type	Day of week	Day of week	Accident descriptions
07/14/201	20:26:39	E MAIN ST / ENGLISH RD	H	ENGLISH RD	H	Accident - No Injuries	Accident - No Injuries	Accident - No Injuries	Accident - No Injuries	Accident - No Injuries	Accident - No Injuries	Accident - No Injuries	Thursday	Thursday	No report taken either happened on private property or damage was less than 500.00 so no report would be taken.
08/01/201	14:20:20	ENGLISH RD / E MAIN ST	MAIN	ENGLISH RD / E MAIN ST	MAIN	Accident - W/Injuries	Accident - W/Injuries	Accident - W/Injuries	Accident - W/Injuries	Accident - W/Injuries	Accident - W/Injuries	Monday	Monday	Rear ended V3 hit V2 into V1 who was stopped at light on English and Main heading eastbound on Main.	

Appendix B—Farmington Police Department Accident Data

08/11/2011	15:53:26	6	ENGLISH RD / E MAIN ST	Accident - No Injuries	Thursday	No report taken either happened on private property or damage was less than 500.00 so no report would be taken.
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Appendix C—Aerial Photographs



Appendix C—Aerial Photographs



Appendix C—Aerial Photographs



Appendix D—Photographs



Looking north on English Rd



Driveways north side of NM 516



Looking north on English Rd



Ramp island in NE corner

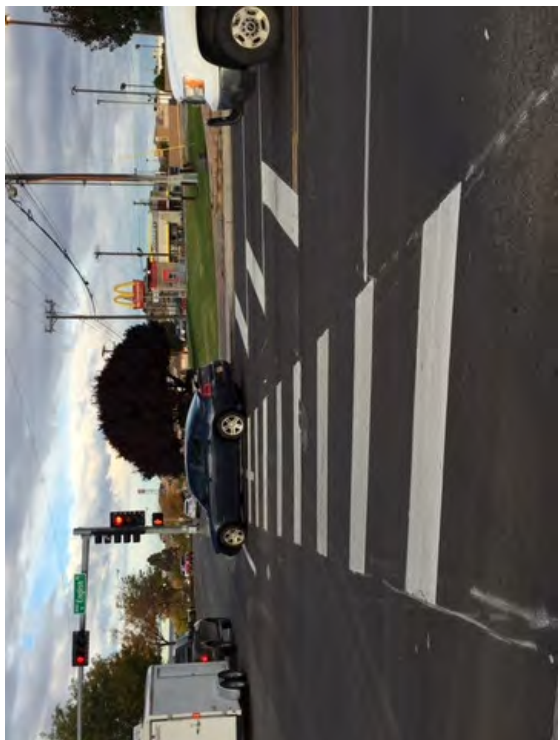
Appendix D—Photographs



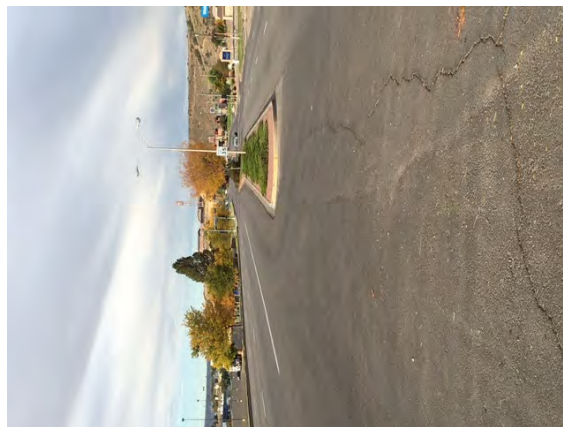
Northeast corner at turn lane



Looking north from southwest corner of intersection



Looking west from the northeast corner of intersection



Looking west on NM 516 at Median Break

Appendix E—Traffic Data



New Mexico Department of Transportation
TIMS ROAD SEGMENTS BY POSTED ROUTE/POINT WITH AADT INFO
NM-ROUTES

As of : 6/6/2016

Route	Traffic Sec Id	Begin Mile	End Mile	FCLS	County Name	AADT					Terminus Year	MHC	
						2015	2014	2013	2012	2011			
NM-509-P	9381	0.000	38.500	B	MC KINLEY	425	440	435	435	2012	L	JCT NM 605 NORTH OF GRANTS, NORTH TO END OF R	25
NM-511-P	9069	0.000	8.170	B	SAN JUAN	506	523	538	538	2011	L	JCT US 64 (FAS1525), NORTH TO THE NEW MEXICO/	41
	24016	8.170	13.900	B	SAN JUAN	625	646	665	665	2011	L	JCT NM 173	41
	9371	13.900	32.346	B		495	512	527	527	2012	L	JCT NM 539 AT NAVAJO DAM	41
NM-512-P	9382	0.000	1.800	B	RIO ARriba	435	450	445	445	2011	L	JCT US64 EAST TO END OF ROUTE AT LOS BRAZOS.	36
	9383	1.800	7.700	B	RIO ARriba	475	492	487	487	2011	L	JCT NM 573	36
NM-513-P	9384	0.000	0.700	B	TORRANCE	68	70	68	1996	2015	L	JCT US90 NORTH TO END OF ROUTE AT ABO MONJUMEN	15
NM-514-P	25969	0.000	1.500	B	RIO ARriba	263	262	259	2015	2015	C	JCT NM 112, NORTH TO JCT US 64 AT LOS OJOS, N	14
NM-515-P	9385	0.000	2.100	B	TAOS	85	88	87	2011	2011	L	JCT NM522 TO THE RED RIVER FISH HATCHERY.	41
NM-516-P	18438	0.000	0.614	B	FRANCO	12,708	12,806	12,671	2009	2009	L	FROM JCT US 64 IN FARMINGTON, NORTHEASTWARD T	11
	35134	0.614	0.802	B	SAN JUAN	22,328	22,148	22,283	2012	2012	L	JCT SOUTHWIDE RIVER ROAD (FL5378)	11
	35136	0.802	1.799	B		20,455	20,561	18,381	2015	2015	C	JCT MORNINGSTAR DRIVE	11
	35136	1.715	1.799	B		20,455	20,561	18,381	2015	2015	C	JCT MORNINGSTAR DRIVE	11
NM-516-M	23647	1.799	2.249	P		18,670	18,520	18,615	2012	2012	L	JCT E. MAIN ST. (FL4357)-90 DEGREE RIGHT-TUR	11
	23647	1.799	2.249	M		18,543	18,394	18,488	2012	2012	L	JCT 20TH STREET (FL4385)	11
NM-516-P	5666	2.249	3.139	P		19,362	19,206	19,305	2012	2012	L	JCT 30TH STREET (FL4386)	11
	5666	2.249	3.139	M		19,991	19,831	19,933	2012	2012	L	JCT 30TH STREET (FL4386)	11
NM-516-P	21314	3.139	3.323	P		18,104	17,958	18,049	2013	2013	L	JCT LARGO STREET (FL5390)	11
	21314	3.139	3.323	M		17,138	17,001	17,089	2013	2013	L	JCT LARGO STREET (FL5390)	11
NM-516-M	35151	3.323	3.672	P		15,489	15,364	15,443	2012	2012	L	JCT SHOPPING CENTER ENTRANCE	11
	35151	3.323	3.672	M		16,276	16,146	16,229	2012	2012	L	JCT SHOPPING CENTER ENTRANCE	11
NM-516-P	35155	3.672	3.857	P		8,835	8,764	8,808	2013	2013	L	JCT ENGLISH ROAD (FL5592)	11
	35155	3.672	3.857	M		7,927	7,863	7,904	2013	2013	L	JCT ENGLISH ROAD (FL5592)	11
NM-516-M	35157	3.857	4.496	P		12,417	12,317	12,380	2011	2011	L	JCT PINON HILLS BOULEVARD (FL4356)	11
	35157	3.857	4.496	M		11,934	11,839	11,900	2011	2011	L	JCT PINON HILLS BOULEVARD (FL4356)	11
NM-516-P	23441	4.496	5.395	P		14,380	13,899	13,770	2015	2015	C	JCT COUNTRY CLUB DRIVE	11
	23441	4.496	5.395	M		13,177	13,247	13,315	2015	2015	C	JCT COUNTRY CLUB DRIVE	11
NM-516-P	35159	5.395	8.140	P		10,605	10,520	10,574	2012	2012	L	JCT COUNTY ROAD 3535	11
	35159	5.395	8.140	M		10,708	10,622	10,676	2012	2012	L	JCT COUNTY ROAD 3535	11
NM-516-M	35159	8.140	8.393	P		10,605	10,520	10,574	2012	2012	L	JCT COUNTY ROAD 3500	11
	35159	8.140	8.393	M		10,708	10,622	10,676	2012	2012	L	JCT COUNTY ROAD 3500	11
NM-516-P	23163	8.393	9.410	P		10,376	10,293	10,159	2014	2014	L	JCT COUNTY ROAD 3500	11
	23163	8.393	9.410	M		10,359	10,276	10,106	2014	2014	L	JCT COUNTY ROAD 3500	11
NM-516-M	23163	9.405	9.959	B		20,735	20,569	20,265	2014	2014	L		11

Run Date : 6/6/2016

TIMS ROAD SEGMENTS BY POSTED ROUTE/POINT WITH AADT INFO

Appendix E—Traffic Data



Lee Engineering, LLC
 Phoenix, Arizona - Dallas, Texas
 Oklahoma City, Oklahoma - San Antonio, Texas
 Albuquerque, New Mexico, United States
 jkouse@lee-eng.com

Count Name: NMC17_01 - Main & English
 Counts
 Site Code:
 Start Date: 09/30/2016
 Page No: 1

Turning Movement Data

Start Time	English Road Southbound					Main Street Westbound					English Road Northbound					Main Street Eastbound					In Total	
	Right	Thru	Left	U-Turn	Peds	Right	Thru	Left	U-Turn	Peds	Right	Thru	Left	U-Turn	Peds	Right	Thru	Left	U-Turn	Peds		
6:00 AM	3	0	2	0	0	1	69	2	0	0	3	1	4	0	0	0	49	1	0	0	0	126
6:15 AM	7	3	2	0	0	1	111	3	0	0	1	2	3	0	0	1	67	6	0	0	0	207
6:30 AM	14	6	8	0	0	3	144	1	0	0	2	7	3	0	0	12	0	78	0	0	1	266
6:45 AM	14	0	2	0	0	4	154	7	0	0	3	4	8	0	0	15	2	103	11	0	0	330
Hourly Total	38	16	14	0	0	8	478	13	0	0	8	14	16	0	0	41	3	287	18	0	1	818
7:00 AM	7	7	9	0	0	6	164	9	0	0	6	10	7	0	0	23	5	78	4	0	0	312
7:15 AM	15	3	4	0	0	11	246	9	1	0	6	26	7	0	0	37	3	126	91	0	0	501
7:30 AM	24	8	7	0	0	17	301	12	0	0	12	22	11	0	0	45	4	198	48	0	0	581
7:45 AM	47	17	23	0	0	44	523	33	1	0	26	65	35	0	0	82	15	521	108	0	0	1042
Hourly Total	37	18	12	0	0	67	11	307	17	2	0	13	8	0	0	29	5	169	11	0	0	618
8:00 AM	24	14	14	0	0	16	224	8	1	0	8	7	0	0	0	19	11	188	15	0	0	504
8:15 AM	16	11	21	0	0	15	195	11	2	0	12	8	9	0	0	29	10	141	11	0	0	462
8:30 AM	21	13	20	0	0	22	217	10	1	0	20	13	8	0	0	42	2	196	10	0	0	519
8:45 AM	90	56	67	0	0	64	543	54	6	0	26	42	32	0	0	100	34	624	47	0	0	1093
Hourly Total	21	9	20	0	0	18	170	16	0	0	20	8	15	0	0	21	12	175	13	0	0	405
9:00 AM	14	15	15	0	0	28	204	23	0	0	19	9	14	0	0	42	2	186	16	0	0	473
9:15 AM	18	21	22	0	0	17	206	11	0	0	15	6	13	0	0	34	17	213	15	0	0	504
9:30 AM	17	21	23	0	0	23	238	23	1	0	17	15	16	0	0	48	17	177	18	0	0	505
9:45 AM	70	66	80	0	0	67	638	73	1	0	59	38	58	0	0	155	48	720	62	0	0	1333
Hourly Total	21	20	20	0	0	25	207	29	1	0	26	11	9	15	0	35	11	203	23	0	0	596
10:00 AM	17	22	25	0	0	26	263	29	0	0	15	14	19	0	0	48	22	216	19	0	0	567
10:15 AM	24	19	27	0	0	34	254	23	0	0	21	19	28	0	0	68	24	224	25	0	0	722
10:30 AM	31	31	33	0	0	24	241	25	0	0	18	14	25	0	0	57	35	244	23	0	0	744
Hourly Total	83	92	105	0	0	109	965	100	1	0	85	66	87	0	0	208	92	887	90	0	0	1969
11:00 AM	22	23	35	0	0	25	261	25	0	0	18	23	29	0	0	70	28	262	20	0	0	510
11:15 AM	37	27	49	0	0	35	274	24	1	0	24	22	30	0	0	76	38	278	30	0	0	636
11:30 AM	28	28	50	0	0	31	283	27	2	0	32	23	39	0	0	79	21	277	26	0	0	632
11:45 AM	47	38	35	0	0	34	288	27	1	0	28	30	27	0	0	85	28	244	25	0	0	842
Hourly Total	134	108	169	0	0	115	1076	103	4	0	128	87	98	125	0	310	105	1051	101	0	0	2387
12:00 PM	34	33	43	0	0	24	300	42	5	0	30	22	20	0	0	72	37	300	23	1	0	571
12:15 PM	34	24	45	0	0	35	261	39	2	0	26	24	41	0	0	91	32	300	20	0	0	563
12:30 PM	34	24	49	0	0	41	300	40	2	0	34	28	39	0	0	102	21	319	21	1	0	564
12:45 PM	47	31	60	0	0	38	302	34	5	0	35	35	45	0	0	115	26	331	36	0	0	683
Hourly Total	149	112	197	0	0	138	1183	155	14	0	140	125	110	145	0	360	116	1240	100	2	0	1478
1:00 PM	37	31	51	0	0	32	292	37	3	0	34	40	33	0	0	98	33	303	34	0	0	639
1:15 PM	36	28	52	0	0	45	277	34	3	0	35	27	41	0	0	104	28	317	33	0	0	578

Appendix E—Traffic Data

1:30 PM	34	17	43	0	1	94	38	207	31	4	0	361	27	32	38	0	0	67	44	287	33	0	0	374	608
1:45 PM	35	25	42	0	1	102	32	255	33	2	0	323	61	29	35	0	0	116	28	308	27	0	0	361	602
Hourly Total	142	101	188	0	1	431	140	1102	136	12	0	1367	137	130	140	0	0	413	131	1235	127	0	0	1463	3124
2:00 PM	34	16	41	0	1	91	26	249	32	1	3	309	26	34	30	0	0	101	26	290	27	0	0	361	602
2:15 PM	34	22	46	0	1	102	40	267	32	1	0	310	32	36	36	0	0	103	14	303	28	0	2	325	600
2:30 PM	42	16	31	0	0	69	36	262	24	2	0	314	38	23	34	0	0	95	20	316	37	1	0	374	672
2:45 PM	31	25	40	0	0	96	37	267	29	1	0	324	23	28	28	0	0	85	29	279	29	0	0	337	643
Hourly Total	141	79	188	0	3	378	141	1014	117	6	3	1277	121	130	144	0	0	365	91	1174	121	1	2	1367	3427
3:00 PM	35	13	36	0	0	84	27	247	26	0	0	300	31	32	27	0	0	90	11	297	48	0	1	367	631
3:15 PM	63	26	49	0	0	158	24	254	22	2	0	302	26	33	30	0	0	82	23	324	40	0	0	367	639
3:30 PM	30	35	36	0	0	110	32	286	32	0	0	361	17	31	26	0	0	73	24	295	29	0	0	342	675
3:45 PM	31	28	42	0	0	102	34	288	41	0	0	361	22	24	20	0	0	69	20	308	32	0	0	363	689
Hourly Total	188	103	163	0	0	464	117	1073	121	2	0	1363	66	130	102	0	0	321	70	1224	144	0	1	1446	3134
4:00 PM	34	26	43	0	0	103	31	280	29	0	0	360	35	37	34	0	0	100	27	286	27	1	4	361	618
4:15 PM	31	36	48	0	0	105	34	280	33	1	0	318	36	38	32	0	0	89	26	327	33	0	1	368	611
4:30 PM	35	27	52	0	0	114	34	248	29	2	0	310	25	33	31	0	0	89	22	327	44	0	1	363	605
4:45 PM	27	30	44	0	0	101	37	265	36	1	0	339	32	26	26	0	0	94	20	333	26	0	1	367	611
Hourly Total	127	109	167	0	0	423	136	1080	127	4	0	1205	131	124	123	0	0	378	106	1203	130	1	7	1533	3140
5:00 PM	34	24	47	0	1	105	26	278	23	0	0	329	46	27	43	0	0	116	25	324	26	0	0	365	625
5:15 PM	31	25	47	0	0	103	31	267	33	1	0	322	30	20	20	0	0	73	25	323	40	0	0	368	606
5:30 PM	38	25	38	0	0	101	27	228	25	1	8	262	30	32	46	0	0	108	18	325	36	0	0	379	670
5:45 PM	40	28	44	0	0	112	36	231	31	1	0	301	25	31	30	0	0	94	15	308	32	0	0	363	663
Hourly Total	143	102	176	0	1	421	124	966	112	3	8	1234	131	110	100	0	0	361	63	1276	144	0	0	1608	3161
6:00 PM	53	25	48	0	0	121	32	227	33	6	0	268	31	35	36	0	0	102	24	266	31	0	0	341	669
6:15 PM	30	23	37	0	0	90	34	223	30	2	0	268	27	25	42	0	0	94	14	269	40	0	0	313	763
6:30 PM	35	27	41	0	0	104	23	254	26	0	0	273	17	34	21	0	0	62	11	287	23	0	0	271	710
6:45 PM	28	24	31	0	0	83	38	215	26	1	0	260	30	28	23	0	0	81	17	244	25	0	0	285	730
Hourly Total	156	99	159	0	0	412	127	888	115	9	0	1136	105	112	122	0	0	339	66	1026	119	0	0	1211	3101
7:00 PM	24	25	34	0	0	83	31	189	24	2	0	248	17	30	26	0	0	73	16	210	24	0	0	262	603
7:15 PM	24	22	34	0	0	82	24	204	19	0	0	247	28	28	29	0	0	66	17	162	23	0	0	232	647
7:30 PM	24	16	34	0	0	74	25	162	21	0	0	198	16	23	29	0	0	70	15	213	29	0	0	267	669
7:45 PM	30	15	32	0	0	78	29	148	10	1	0	188	12	24	27	0	0	63	15	198	20	0	0	233	662
Hourly Total	102	79	136	0	0	317	109	682	74	3	0	678	76	106	111	0	0	292	65	613	96	0	0	674	2461
8:00 PM	45	16	24	0	0	85	26	129	11	0	0	166	17	25	20	0	0	62	16	162	26	0	0	227	540
8:15 PM	30	22	32	0	0	82	19	127	11	5	0	162	22	21	29	0	0	72	15	166	10	0	0	191	637
8:30 PM	25	14	29	0	0	69	21	119	13	2	0	155	22	25	28	0	0	75	11	154	21	0	0	189	482
8:45 PM	25	9	20	0	0	65	19	105	16	1	0	141	11	19	15	0	0	45	16	140	27	1	0	164	425
Hourly Total	122	61	105	0	0	266	66	480	51	8	0	624	72	80	62	0	0	254	69	642	67	1	0	788	1954
9:00 PM	6	10	26	0	0	42	17	111	3	0	0	131	26	21	20	0	0	67	5	102	15	0	0	180	420
9:15 PM	6	7	12	0	0	28	16	81	7	1	0	107	21	27	21	0	0	65	13	132	15	0	0	160	364
9:30 PM	10	5	20	0	0	35	17	95	5	0	0	118	16	20	20	0	0	56	4	105	6	0	0	117	328
9:45 PM	13	3	12	0	0	39	12	79	5	1	0	97	6	8	9	0	0	24	1	107	6	0	0	116	265
Hourly Total	36	25	70	0	0	133	64	367	20	2	0	463	69	77	70	0	0	216	23	504	46	0	0	673	1376
10:00 PM	9	6	13	0	0	28	8	67	4	0	0	79	5	4	13	0	0	22	9	92	10	0	0	111	240
10:15 PM	4	5	7	0	0	16	5	69	3	0	0	66	6	7	6	0	0	21	3	65	6	0	0	96	199
10:30 PM	10	2	7	0	0	19	3	49	4	0	0	55	2	6	11	0	0	19	8	74	7	0	0	89	182
10:45 PM	3	2	6	0	0	11	4	34	1	0	0	36	5	0	12	0	0	17	3	63	6	0	0	62	129
Hourly Total	26	15	33	0	0	74	20	207	12	0	0	239	16	17	44	0	0	79	23	304	31	0	0	368	700
11:00 PM	3	1	4	0	0	2	3	49	0	0	0	52	7	4	6	0	0	17	2	63	3	0	0	66	136
11:15 PM	3	10	10	0	0	23	4	31	2	0	0	37	5	5	4	0	0	14	3	40	5	0	0	48	122
11:30 PM	4	2	3	0	0	8	2	25	1	0	0	28	0	4	4	0	0	8	4	48	2	0	0	55	103
11:45 PM	2	1	2	0	0	5	4	29	1	0	0	34	2	1	2	0	0	5	1	42	2	1	1	46	90

Appendix E—Traffic Data

10:15 AM	20	15	31	0	1	68	25	223	27	1	0	278	16	14	23	0	0	55	21	200	28	0	0	248	646
10:30 AM	23	18	20	0	0	61	26	227	36	0	0	262	21	6	19	0	0	46	12	215	27	0	0	264	653
10:45 AM	22	23	32	0	0	77	32	230	28	2	0	262	16	16	15	0	0	52	26	245	24	0	0	265	716
Hourly Total	94	70	111	0	2	204	109	680	118	4	0	1129	76	60	77	0	0	203	77	637	96	0	0	1010	2428
11:00 AM	24	15	31	0	0	70	32	254	38	0	3	284	21	22	10	0	2	61	24	265	23	0	0	312	737
11:15 AM	34	30	40	0	0	94	36	242	26	3	0	269	19	15	23	0	1	67	24	248	27	0	1	268	748
11:30 AM	27	19	33	0	0	79	28	291	44	1	0	324	31	15	27	0	0	73	25	244	24	0	0	263	709
11:45 AM	41	26	34	0	0	101	36	279	44	2	0	364	37	27	26	0	0	103	22	262	34	0	0	318	885
Hourly Total	126	81	138	0	0	344	127	966	152	6	3	1281	108	79	107	0	3	264	94	1019	108	0	1	1221	3140
12:00 PM	40	20	41	0	0	109	36	244	41	2	0	322	33	20	37	0	0	90	29	265	32	0	0	316	837
12:15 PM	33	20	31	0	3	84	29	273	35	7	0	344	26	19	34	0	0	81	26	321	25	0	0	374	883
12:30 PM	44	26	36	0	0	108	36	264	40	0	0	343	26	14	34	0	1	74	26	254	21	0	0	311	834
12:45 PM	37	21	45	0	0	103	34	270	38	4	0	346	24	24	23	0	1	71	23	269	33	0	0	345	866
Hourly Total	154	85	153	0	3	402	137	1051	154	13	0	1359	111	77	120	0	1	316	105	1119	121	0	0	1345	3419
1:00 PM	30	28	48	0	4	106	41	265	34	1	1	341	31	28	30	0	4	89	26	301	29	0	2	358	884
1:15 PM	36	27	40	0	0	103	39	254	33	0	0	328	30	34	45	0	0	118	23	260	48	1	0	362	900
1:30 PM	34	25	37	0	0	95	42	264	41	0	0	347	29	26	35	0	0	90	29	312	33	0	0	374	916
1:45 PM	46	33	37	0	0	116	33	267	33	1	0	364	37	25	32	0	0	94	21	263	30	0	0	344	918
Hourly Total	146	113	162	0	4	421	155	1080	141	2	1	1378	136	122	142	0	4	400	101	1136	140	1	2	1438	3627
2:00 PM	30	33	39	0	0	110	37	261	38	0	0	326	36	25	31	0	0	91	25	308	34	1	0	368	915
2:15 PM	26	31	48	0	0	107	36	265	36	3	0	340	31	30	32	0	2	93	29	307	32	0	2	369	908
2:30 PM	26	16	41	0	0	85	26	207	34	2	0	278	40	25	36	0	0	101	24	265	26	0	2	265	689
2:45 PM	41	31	45	0	0	118	36	269	33	0	0	328	34	31	29	0	1	94	31	266	37	0	0	364	884
Hourly Total	145	111	174	0	0	430	144	992	141	5	0	1272	140	111	120	0	3	379	119	1226	129	1	4	1476	3656
3:00 PM	26	30	38	0	0	94	36	254	39	0	0	309	30	30	31	0	0	91	24	272	35	0	0	331	825
3:15 PM	27	24	42	0	0	93	34	245	31	3	0	313	40	36	35	0	0	110	27	265	34	0	0	346	862
3:30 PM	40	27	38	0	0	105	32	254	35	0	0	291	33	33	44	0	0	110	26	274	35	0	0	341	847
3:45 PM	32	31	54	0	0	117	32	215	41	2	0	290	27	40	45	0	0	112	24	265	31	0	0	320	838
Hourly Total	125	112	172	0	0	409	134	918	146	5	0	1269	130	139	165	0	0	423	103	1036	139	0	0	1338	3373
4:00 PM	40	21	32	0	0	83	29	223	42	1	0	294	26	34	33	0	0	95	24	264	31	1	3	303	803
4:15 PM	31	25	43	0	0	89	39	212	32	6	0	269	28	35	21	0	0	84	20	269	37	0	0	316	789
4:30 PM	32	25	47	0	1	100	36	250	29	5	0	269	37	25	31	0	0	93	17	221	39	0	0	277	707
4:45 PM	39	27	34	0	0	100	25	213	28	1	0	265	23	31	40	0	0	94	20	218	33	0	1	271	730
Hourly Total	142	102	156	0	1	400	129	867	128	13	0	1137	117	125	125	0	0	367	81	962	140	1	4	1164	3088
5:00 PM	36	27	43	0	0	106	25	188	30	4	0	268	31	36	29	0	0	95	17	227	38	0	3	272	732
5:15 PM	36	26	40	0	0	106	31	204	29	0	0	264	26	33	33	0	0	101	15	246	27	0	0	268	709
5:30 PM	37	19	39	0	0	85	34	194	35	2	0	265	26	27	31	0	0	87	13	221	30	0	0	264	721
5:45 PM	30	15	45	0	0	90	27	219	33	4	0	263	31	39	35	0	0	105	11	227	24	0	0	262	740
Hourly Total	141	89	167	0	0	397	117	815	127	10	0	1070	136	135	138	0	0	369	56	921	139	0	3	1086	2852
6:00 PM	26	26	39	0	0	83	28	174	26	2	1	291	26	26	21	0	0	71	21	256	28	2	0	307	702
6:15 PM	31	18	22	0	0	71	30	188	32	0	0	261	23	42	29	0	0	94	15	212	28	0	0	255	671
6:30 PM	26	21	29	0	0	76	27	166	30	3	0	226	31	30	32	0	0	83	21	188	24	0	0	241	636
6:45 PM	32	18	34	0	0	84	34	188	20	2	0	222	17	22	30	0	0	69	12	176	20	0	0	239	583
Hourly Total	117	63	124	0	0	324	120	665	109	7	1	930	96	119	112	0	0	327	69	641	100	2	0	1011	2560
7:00 PM	27	27	31	0	0	81	32	188	27	2	2	230	15	25	20	0	2	60	17	212	21	0	1	260	621
7:15 PM	25	16	40	0	1	81	21	169	17	1	3	208	22	17	21	0	0	60	17	187	26	0	0	230	679
7:30 PM	19	23	20	0	1	70	28	167	18	2	0	216	20	20	24	0	0	72	24	161	24	0	0	243	601
7:45 PM	24	10	38	0	0	72	28	169	16	1	0	204	25	16	21	0	0	62	13	143	28	1	0	195	629
Hourly Total	91	76	137	0	2	304	110	654	78	6	5	828	90	76	86	0	2	254	71	737	99	1	1	938	2324
Grand Total	3349	2329	3762	0	31	9400	3107	20592	2937	157	35	31639	2623	2175	2947	1	27	8146	2191	20796	2803	13	30	31893	81162
Approach %	315	241	261	0	0	91	91	801	90	0	0	302	31	6	26	0	0	100	6	84	92	0	0	100	100
Total %	4.1	2.9	4.6	0.0	0.0	11.6	3.8	31.5	3.5	0.2	0.2	38.0	3.2	3.2	3.6	0.0	0.0	10.0	2.7	33.0	3.6	0.0	0.0	38.3	8.7

Appendix E—Traffic Data

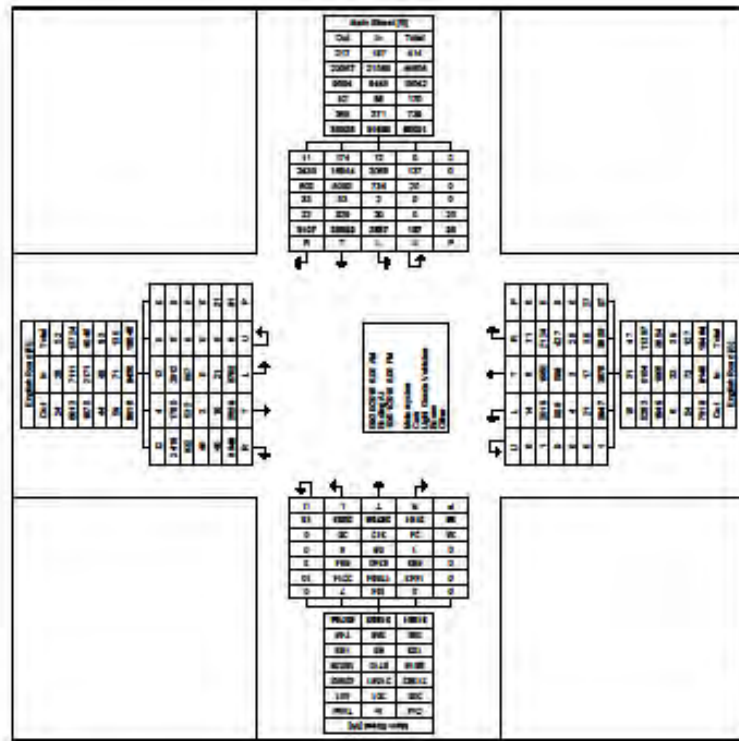
Motorcycles	12	4	1.2	0	28	11	174	1.2	0	197	11	6	14	0	31	0	194	7	0	201	467
% Motorcycles	0.4	0.2	0.3	-	0.3	0.4	0.7	0.4	0.0	0.6	0.4	0.2	0.5	0.0	0.4	0.0	0.7	0.2	0.0	0.6	0.6
Cars	2419	1760	2812	0	7111	2439	16944	2066	137	21066	2124	1900	2019	1	6104	1443	17664	2214	10	21951	50365
% Cars	72.2	76.4	77.6	-	76.4	78.5	66.2	72.9	67.3	66.1	61.0	76.1	66.5	100.0	74.0	67.1	66.9	75.5	76.9	-	69.4
Light Goods Vehicles	632	532	607	0	2171	632	6062	734	20	6449	427	595	669	0	1929	633	6340	694	3	6710	20254
% Light Goods Vehicles	2.48	22.8	21.5	-	23.0	19.4	31.6	26.9	12.7	26.0	16.3	22.9	36.2	0.0	23.4	21.8	31.1	23.3	23.1	-	30.4
Buses	46	3	0	0	46	26	63	2	0	66	26	3	4	0	33	1	5.6	6	0	-	66
% Buses	1.4	0.1	0.0	-	0.6	1.1	0.2	0.1	0.0	0.3	1.0	0.1	0.1	0.0	0.4	0.0	0.2	0.3	0.0	-	0.2
Single-Unit Trucks	20	9	14	0	43	16	202	19	0	267	33	16	16	0	64	11	223	11	0	-	245
% Single-Unit Trucks	0.6	0.4	0.4	-	0.6	0.5	0.6	0.7	0.0	0.9	1.3	0.6	0.5	0.0	0.0	0.5	0.6	0.4	0.0	-	0.8
Articulated Trucks	19	1	7	0	27	8	97	1	0	104	1	1	8	0	8	12	89	9	0	-	110
% Articulated Trucks	0.6	0.0	0.2	-	0.3	0.2	0.4	0.0	0.0	0.3	0.0	0.0	0.2	0.0	0.1	0.6	0.3	0.3	0.0	-	0.3
Bicycles on Road	1	0	0	0	1	0	0	0	0	0	1	0	0	0	1	1	0	0	0	-	1
% Bicycles on Road	0.0	0.0	0.0	-	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	-	0.0
Bicycles on Crosswalk	-	-	-	2	-	-	-	-	0	-	-	-	-	-	2	-	-	-	-	-	2
% Bicycles on Crosswalk	-	-	-	6.0	-	-	-	0.0	-	-	-	-	-	7.4	-	-	-	-	-	-	6.3
PeDESTRIANS	-	-	-	-	29	-	-	-	26	-	-	-	-	-	26	-	-	-	-	-	26
% PeDESTRIANS	-	-	-	-	0.31	-	-	-	0.26	-	-	-	-	-	0.26	-	-	-	-	-	0.27

Appendix E—Traffic Data



Lee Engineering, LLC
 Phoenix, Arizona - Dallas, Texas
 Oklahoma City, Oklahoma - San Antonio, Texas
 Albuquerque, New Mexico, United States
 lee@lee-eng.com

Count Name: NIM217_01 - Main & English
 Counts
 Site Code:
 Start Date: 09/30/2016
 Page No. 6



Turning Movement Data Plot

Appendix E—Traffic Data



Lee Engineering, LLC
 Phoenix, Arizona - Dallas, Texas
 Oklahoma City, Oklahoma - San Antonio, Texas
 Albuquerque, New Mexico, United States
 julius@lee-eng.com

Count Name: NIM217.01 - Main & English
 Counts
 Site Code:
 Start Date: 09/30/2016
 Page No.: 7

Turning Movement Peak Hour Data (11:00 AM)

Start Time	English Road Southbound					Main Street Westbound					English Road Northbound					Main Street Eastbound												
	Right	Thru	Left	U-Turn	Peaks	Right	Thru	Left	U-Turn	Peaks	Right	Thru	Left	U-Turn	Peaks	Right	Thru	Left	U-Turn	Peaks	Right	Thru	Left	U-Turn	Peaks	App. Total	HL Total	
11:00 AM	22	23	35	0	1	80	25	261	25	0	0	301	18	23	29	0	0	0	0	0	70	28	262	20	0	1	310	761
11:15 AM	37	27	49	0	1	113	25	274	24	1	0	324	24	22	30	0	1	76	28	270	30	0	1	336	848	848	848	
11:30 AM	26	28	50	0	0	106	31	263	27	2	0	323	17	23	36	0	0	79	21	277	26	0	0	324	832	832	832	
11:45 AM	47	20	35	0	1	110	34	288	27	1	1	350	28	30	27	0	0	86	28	244	25	0	1	287	842	842	842	
Total	134	106	169	0	3	409	115	1076	103	4	1	1286	87	98	125	0	1	310	105	1061	101	0	0	1267	3264	3264	3264	
Approach %	32.8	25.9	41.3	0.0	-	-	8.9	62.9	7.9	0.2	-	-	28.1	31.6	40.3	0.0	-	-	8.3	63.7	8.0	0.0	-	-	-	-	-	
Total %	4.1	3.2	5.1	0.0	-	12.5	3.5	32.6	3.1	0.1	-	38.5	2.6	3.0	3.8	0.0	-	8.4	3.2	32.3	3.1	0.0	-	-	38.6	866	866	
PF/F	0.713	0.946	0.845	0.000	-	0.905	0.948	0.854	0.954	0.900	-	0.827	0.777	0.817	0.801	0.000	-	0.972	0.938	0.954	0.942	0.000	-	-	0.943	0.967	0.967	
Motorcycles	0	0	0	0	-	1	1	3	1	0	-	6	1	0	1	0	-	2	0	4	1	0	-	-	6	13	13	
% Motorcycles	0.0	0.9	0.0	-	-	0.2	0.9	0.3	1.0	0.0	-	0.4	1.1	0.0	0.8	-	-	0.6	0.0	0.4	1.0	-	-	-	0.4	0.4		
Cars	95	83	119	0	-	297	90	696	61	3	-	820	62	69	82	0	-	212	70	647	76	0	-	-	785	2124		
% Cars	70.9	78.3	70.4	-	-	72.6	78.3	61.9	58.2	75.0	-	63.2	71.3	68.4	65.6	-	-	60.4	69.7	61.0	77.2	-	-	-	62.7	64.7		
Light Goods Vehicle	34	21	48	0	-	103	22	381	41	1	-	445	19	30	41	0	-	90	33	388	20	0	-	-	441	1079		
% Light Goods Vehicle	25.4	19.6	28.4	-	-	25.2	19.1	26.4	38.0	26.0	-	34.3	21.8	30.6	32.0	-	-	29.0	31.4	36.6	19.8	-	-	-	34.8	32.6		
Buses	1	0	0	0	-	1	1	12	0	0	-	13	2	0	0	0	-	2	0	3	0	0	-	-	3	19		
% Buses	0.7	0.0	0.0	-	-	0.2	0.9	1.1	0.0	0.0	-	1.0	2.3	0.0	0.0	-	-	0.6	0.0	0.3	0.0	-	-	-	0.2	0.6		
Single Unit Trucks	2	1	2	0	-	5	1	12	0	0	-	19	3	0	1	0	-	4	0	14	1	0	-	-	16	37		
% Single Unit Trucks	1.6	0.9	1.2	-	-	1.2	0.8	1.1	0.0	0.0	-	1.0	3.4	0.0	0.8	-	-	1.3	0.0	1.3	1.0	-	-	-	1.2	1.1		
Articulated Trucks	2	0	0	0	-	2	0	2	0	0	-	2	0	0	0	0	-	0	2	5	1	0	-	-	8	12		
% Articulated Trucks	1.5	0.0	0.0	-	-	0.5	0.0	0.2	0.0	0.0	-	0.2	0.0	0.0	0.0	-	-	0.0	1.8	0.5	1.0	-	-	-	0.6	0.4		
Bicycles on Road	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	0	0	0	0		
% Bicycles on Road	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	0.0	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	-	0.0	0.0		
Bicycles on Crosswalk	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	-	-	0	-	
% Bicycles on Crosswalk	-	-	-	-	-	0.0	-	-	-	-	-	0.0	-	-	-	-	-	0.0	-	-	-	-	-	-	-	0.0	-	
PeDESTRIANS	-	-	-	-	-	3	-	-	-	-	-	3	-	-	-	-	-	1	-	-	-	-	-	-	-	3	-	
% PeDESTRIANS	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	-	-	-	100.0	-	

Appendix E—Traffic Data



Lee Engineering, LLC
 Phoenix, Arizona - Dallas, Texas
 Oklahoma City, Oklahoma - San Antonio, Texas
 Albuquerque, New Mexico, United States
 jku@lee-eng.com

Count Name: NIM217.01 - Main & English
 Counts
 Site Code:
 Start Date: 09/30/2016
 Page No. 9

Turning Movement Peak Hour Data (12:30 PM)

Start Time	English Road Southbound					Main Street Westbound					English Road Northbound					Main Street Eastbound													
	Right	Thru	Left	U-Turn	Peds	Right	Thru	Left	U-Turn	Peds	Right	Thru	Left	U-Turn	Peds	Right	Thru	Left	U-Turn	Peds	Right	Thru	Left	U-Turn	Peds	App. Total	HL Total		
12:30 PM	34	24	49	0	0	107	41	300	40	2	0	383	34	29	38	0	0	102	21	319	21	1	0	362	654				
12:45 PM	47	31	60	0	1	138	38	302	34	5	2	379	35	35	45	0	0	115	26	331	36	0	0	363	1025				
1:00 PM	37	31	51	0	0	118	32	282	37	3	0	364	23	40	33	0	0	96	33	303	34	0	0	370	930				
1:15 PM	35	28	52	0	0	115	45	277	34	3	0	359	36	27	41	0	0	104	28	317	33	0	0	378	907				
Total	154	114	212	0	1	480	156	1161	145	13	2	1475	126	121	158	0	0	417	108	1270	124	1	0	1403	3875				
Approach %	32.1	23.8	44.2	0.0	0.0	100.0	10.6	78.7	98.0	0.9	0.0	100.0	30.7	31.4	37.9	0.0	0.0	7.2	84.5	8.3	0.1	0.0	0.0	0.0	38.8	100.0			
Total %	4.0	2.9	5.5	0.0	0.0	12.4	4.0	30.0	3.7	0.3	0.0	38.1	3.3	3.4	4.1	0.0	0.0	1.8	2.8	32.8	3.2	0.0	0.0	0.0	38.8	100.0			
PFV	0.819	0.919	0.883	0.000	0.000	0.670	0.867	0.951	0.906	0.690	0.000	0.863	0.888	0.919	0.878	0.000	0.000	0.907	0.818	0.959	0.981	0.250	0.000	0.000	0.966	0.945			
Motorcycles	1	0	0	0	0	1	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	6	15		
% Motorcycles	0.6	0.0	0.0	0.0	0.0	0.2	0.0	0.7	0.0	0.0	0.0	0.5	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.5	0.0	0.0	0.0	0.0	0.4	0.4			
Cars	103	84	169	0	0	265	117	760	102	9	0	869	112	103	121	0	0	336	78	838	91	0	0	0	865	2674			
% Cars	66.9	73.7	78.2	0.0	0.0	74.0	75.0	66.5	70.3	68.2	0.0	67.0	67.5	78.6	76.6	0.0	0.0	80.6	72.2	65.0	73.4	0.0	0.0	0.0	66.2	69.0			
Light Goods Vehicle	46	28	42	0	0	117	35	378	41	4	0	456	13	28	35	0	0	76	30	417	31	1	0	0	479	1128			
% Light Goods Vehicle	29.9	25.4	19.8	0.0	0.0	24.4	22.4	32.4	28.3	30.0	0.0	30.9	10.2	21.4	22.2	0.0	0.0	18.2	27.8	32.8	25.0	100.0	0.0	0.0	31.9	29.1			
Buses	1	0	0	0	0	1	2	1	0	0	0	3	0	0	1	0	0	1	0	2	0	0	0	0	2	7			
% Buses	0.6	0.0	0.0	0.0	0.0	0.2	1.3	0.1	0.0	0.0	0.0	0.2	0.0	0.0	0.0	0.0	0.0	0.2	0.0	0.2	0.0	0.0	0.0	0.0	0.1	0.2			
Single Unit Trucks	1	1	2	0	0	4	2	12	2	0	0	16	2	0	0	0	0	2	0	12	2	0	0	0	14	36			
% Single Unit Trucks	0.6	0.9	0.9	0.0	0.0	0.8	1.3	1.0	1.4	0.0	0.0	1.1	1.6	0.0	0.0	0.0	0.0	0.5	0.0	0.9	1.6	0.0	0.0	0.0	0.9	0.9			
Articulated Trucks	2	0	0	0	0	2	0	4	0	0	0	4	0	0	1	0	0	1	0	7	0	0	0	0	7	14			
% Articulated Trucks	1.3	0.0	0.0	0.0	0.0	0.4	0.0	0.3	0.0	0.0	0.0	0.3	0.0	0.0	0.6	0.0	0.0	0.2	0.0	0.6	0.0	0.0	0.0	0.0	0.5	0.4			
Bicycles on Road	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	1	0	0	0	0	0	0	0	1			
% Bicycles on Road	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.8	0.0	0.0	0.0	0.0	0.2	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0			
Bicycles on Crosswalk	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0			
% Bicycles on Crosswalk	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0			
Pedestrians	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0			
% Pedestrians	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0			

Appendix E—Traffic Data



Lee Engineering, LLC
 Phoenix, Arizona - Dallas, Texas
 Oklahoma City, Oklahoma - San Antonio, Texas
 Albuquerque, New Mexico, United States
 julius@lee-eng.com

Count Name: NIM217_01 - Main & English
 Counts
 Site Code:
 Start Date: 09/30/2016
 Page No.: 11

Turning Movement Peak Hour Data (11:00 AM)

Start Time	English Road Southbound					Main Street Westbound					English Road Northbound					Main Street Eastbound						
	Right	Thru	Left	U-Turn	Peds	Right	Thru	Left	U-Turn	Peds	Right	Thru	Left	U-Turn	Peds	Right	Thru	Left	U-Turn	Peds	App. Total	
11:00 AM	24	15	31	0	0	32	224	38	0	3	294	21	22	18	0	24	265	25	0	0	312	
11:15 AM	34	20	40	0	0	28	242	25	3	0	269	19	15	23	0	57	23	240	27	0	1	280
11:30 AM	27	19	33	0	0	26	281	44	1	0	324	31	15	27	0	73	25	244	24	0	0	293
11:45 AM	41	28	34	0	0	28	279	44	2	0	364	37	27	26	0	103	22	262	34	0	0	318
Total	126	80	138	0	0	127	995	162	6	3	1381	108	76	107	0	294	94	1019	106	0	0	1221
Approach %	36.6	23.3	40.1	0.0	0.0	9.9	77.8	11.9	0.5	0.2	40.8	3.4	25	3.4	0.0	8.4	7.7	83.6	8.8	0.0	0.0	38.9
Total %	4.0	2.5	4.4	0.0	0.0	4.0	31.7	4.8	0.2	0.2	40.8	0.7	0.3	0.3	0.0	8.4	0.9	10.1	1.1	0.0	0.0	38.9
PF/F	0.788	0.768	0.823	0.000	0.000	0.814	0.892	0.864	0.900	0.863	0.730	0.731	0.888	0.900	0.000	0.714	0.940	0.961	0.794	0.000	0.000	0.960
Motorcycles	0	0	0	0	0	0	16	0	0	0	16	0	0	0	0	0	0	12	0	0	0	12
% Motorcycles	0.0	0.0	0.0	0.0	0.0	0.0	1.6	0.0	0.0	0.0	1.2	0.0	0.0	0.0	0.0	0.0	0.0	1.2	0.0	0.0	0.0	1.0
Cars	83	64	104	0	0	102	868	110	3	0	873	85	86	89	0	210	89	638	81	0	0	776
% Cars	65.9	80.0	75.4	0.0	0.0	80.3	86.1	72.4	90.0	0.0	63.1	78.7	70.9	84.5	0.0	71.4	82.8	63.4	75.0	0.0	0.0	63.6
Light Goods Vehicle	40	15	34	0	0	24	317	41	3	0	365	21	22	38	0	81	36	265	27	0	0	427
% Light Goods Vehicle	31.7	18.8	24.6	0.0	0.0	18.9	31.8	27.0	90.0	0.0	26.5	19.4	27.8	36.5	0.0	27.8	37.2	26.8	26.0	0.0	0.0	31.3
Buses	1	1	0	0	0	1	0	0	0	0	1	2	0	0	0	2	0	0	0	0	0	0
% Buses	0.8	1.3	0.0	0.0	0.0	0.8	0.0	0.0	0.0	0.0	0.1	1.8	0.0	0.0	0.0	0.7	0.0	0.0	0.0	0.0	0.0	0.2
Single-Unit Trucks	1	0	0	0	0	0	5	1	0	0	6	0	1	0	0	1	0	5	0	0	0	6
% Single-Unit Trucks	0.8	0.0	0.0	0.0	0.0	0.0	0.5	0.7	0.0	0.0	0.5	0.0	1.3	0.0	0.0	0.3	0.0	0.5	0.0	0.0	0.0	0.4
Articulated Trucks	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	1
% Articulated Trucks	0.8	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.1	0.0	0.0	0.0	0.1
Bicycles on Road	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
% Bicycles on Road	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Bicycles on Crosswalk	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
% Bicycles on Crosswalk	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
PeDESTRIANS	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
% PeDESTRIANS	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

Appendix E—Traffic Data



Lee Engineering, LLC
 Phoenix, Arizona - Dallas, Texas
 Oklahoma City, Oklahoma - San Antonio, Texas
 Albuquerque, New Mexico, United States
 jku@lee-eng.com

Count Name: NIM217_01 - Main & English
 Counts
 Site Code:
 Start Date: 09/30/2016
 Page No: 13

Turning Movement Peak Hour Data (1:15 PM)

Start Time	English Road Southbound				Main Street Westbound				English Road Northbound				Main Street Eastbound				N. Total									
	Right	Thru	Left	U-Turn	Right	Thru	Left	U-Turn	Right	Thru	Left	U-Turn	Right	Thru	Left	U-Turn										
1:15 PM	36	27	40	0	0	103	38	254	33	0	0	326	38	34	46	0	118	23	260	46	1	0	362	900		
1:30 PM	34	26	37	0	0	96	42	264	41	0	0	347	29	35	35	0	99	29	312	33	0	0	374	916		
1:45 PM	46	33	37	0	0	116	39	267	33	1	0	364	37	26	32	0	94	21	269	30	0	0	344	918		
2:00 PM	38	33	39	0	0	110	37	251	38	0	0	326	35	25	31	0	91	25	326	34	1	0	388	915		
Total	154	118	153	0	0	425	151	1096	145	1	0	1363	140	119	143	0	402	96	1223	145	2	0	1469	3660		
Approach %	36.2	27.8	36.0	0.0	-	-	11.1	79.2	10.6	0.1	-	-	34.8	29.6	36.6	0.0	-	6.7	83.3	99	0.1	-	-	-	-	
Total %	4.2	3.2	4.2	0.0	-	11.6	4.1	28.1	4.0	0.0	-	37.3	3.8	3.3	3.9	0.0	-	11.0	2.7	23.4	4.0	0.1	-	40.1	-	
P/P	0.837	0.934	0.956	0.000	-	0.916	0.899	0.917	0.894	0.250	-	0.898	0.897	0.850	0.794	0.000	-	0.862	0.845	0.832	0.765	0.900	-	0.949	0.906	
Motorcycles	1	0	0	0	-	1	0	9	3	0	-	12	2	0	2	0	-	4	0	6	0	0	-	6	23	
% Motorcycles	0.6	0.0	0.0	-	-	0.2	0.0	0.8	2.1	0.0	-	0.8	1.4	0.0	1.4	-	-	1.0	0.0	0.5	0.0	0.0	-	0.4	0.6	
Cars	150	92	124	0	-	388	114	738	109	1	-	963	109	99	100	0	-	205	83	706	111	1	-	971	2995	
% Cars	77.9	78.0	81.0	-	-	79.1	75.5	68.3	75.2	100.0	-	70.7	77.9	72.3	69.9	-	-	73.4	64.3	65.1	76.6	90.0	-	66.1	70.1	
Light Goods Vehicles	30	36	20	0	-	94	36	314	33	0	-	383	27	33	40	0	-	100	34	416	34	1	-	486	1002	
% Light Goods Vehicles	18.5	22.0	16.3	-	-	19.9	23.6	28.5	22.0	0.0	-	28.1	19.3	27.7	28.0	-	-	24.9	34.7	34.0	23.4	90.0	-	33.0	28.9	
Buses	1	0	0	0	-	1	1	1	0	0	-	2	1	0	0	0	-	1	0	1	0	0	-	1	5	
% Buses	0.6	0.0	0.0	-	-	0.2	0.7	0.1	0.0	0.0	-	0.1	0.7	0.0	0.0	-	-	0.2	0.0	0.1	0.0	0.0	-	0.1	0.1	
Single-Unit Trucks	1	0	0	0	-	1	0	3	0	0	-	3	1	0	1	0	-	2	1	4	0	0	-	6	11	
% Single-Unit Trucks	0.6	0.0	0.0	-	-	0.2	0.0	0.3	0.0	0.0	-	0.2	0.7	0.0	0.7	-	-	0.5	1.0	0.3	0.0	0.0	-	0.3	0.3	
Articulated Trucks	1	0	1	0	-	2	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	2	
% Articulated Trucks	0.6	0.0	0.7	-	-	0.5	0.0	0.0	0.0	0.0	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	0.0	-	0.0	0.1	
Bicycles on Road	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	
% Bicycles on Road	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	0.0	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	0.0	-	0.0	0.0	
Bicycles on Crosswalk	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	-	-	-	-	-	-	-	-	-
% Bicycles on Crosswalk	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	
Pedestrians	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	-	-	-	-	-	-	-	-	
% Pedestrians	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	



























Appendix F—Traffic Data Analysis



Appendix F—Traffic Data Analysis

HCM 2010 Signalized Intersection Summary 3: English Rd & E Main St - Max 2

10/28/2016

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		 			 						 	
Volume (veh/h)	124	1270	108	145	1161	156	158	131	128	212	114	154
Number	1	6	16	5	2	12	3	8	18	7	4	14
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj Sat Flow, veh/h/ln	1881	1881	1900	1881	1881	1881	1881	1881	1881	1881	1881	1881
Adj Flow Rate, veh/h	135	1380	117	158	1262	0	172	142	0	230	124	167
Adj No. of Lanes	1	3	0	1	3	1	1	1	1	1	1	1
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	1	1	1	1	1	1	1	1	1	1	1	1
Cap, veh/h	328	1789	152	142	2622	816	309	200	170	314	233	198
Arrive On Green	0.06	0.37	0.37	0.20	0.51	0.00	0.10	0.11	0.00	0.12	0.12	0.12
Sat Flow, veh/h	1792	4824	409	358	5136	1599	1792	1881	1599	1792	1881	1599
Grp Volume(v), veh/h	135	979	518	158	1262	0	172	142	0	230	124	167
Grp Sat Flow(s),veh/h/ln	1792	1712	1809	358	1712	1599	1792	1881	1599	1792	1881	1599
Q Serve(g_s), s	5.5	29.9	29.9	24.0	18.9	0.0	10.0	8.7	0.0	13.6	7.3	12.1
Cycle Q Clear(g_c), s	5.5	29.9	29.9	24.0	18.9	0.0	10.0	8.7	0.0	13.6	7.3	12.1
Prop In Lane	1.00		0.23	1.00		1.00	1.00		1.00	1.00		1.00
Lane Grp Cap(c), veh/h	328	1270	671	142	2622	816	309	200	170	314	233	198
V/C Ratio(X)	0.41	0.77	0.77	1.12	0.48	0.00	0.56	0.71	0.00	0.73	0.53	0.84
Avail Cap(c_a), veh/h	578	1270	671	142	2622	816	340	460	391	314	460	391
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	20.6	32.9	32.9	29.2	18.8	0.0	41.4	51.2	0.0	41.3	48.8	50.9
Incr Delay (d2), s/veh	0.3	4.6	8.4	110.2	0.6	0.0	0.7	1.7	0.0	7.5	0.7	3.8
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	2.7	15.0	16.5	4.3	9.0	0.0	5.0	4.6	0.0	7.3	3.9	5.6
LnGrp Delay(d),s/veh	20.9	37.5	41.3	139.4	19.5	0.0	42.0	53.0	0.0	48.8	49.5	54.6
LnGrp LOS	C	D	D	F	B		D	D		D	D	D
Approach Vol, veh/h		1632			1420			314			521	
Approach Delay, s/veh		37.3			32.8			47.0			50.8	
Approach LOS		D			C			D			D	
Timer	1	2	3	4	5	6	7	8				
Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	13.4	66.6	18.0	20.7	30.0	50.0	20.0	18.6				
Change Period (Y+Rc), s	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0				
Max Green Setting (Gmax), s	24.0	44.0	14.0	29.0	24.0	44.0	14.0	29.0				
Max Q Clear Time (g_c+I1), s	7.5	20.9	12.0	14.1	26.0	31.9	15.6	10.7				
Green Ext Time (p_c), s	0.0	9.2	0.0	0.5	0.0	6.7	0.0	0.6				
Intersection Summary												
HCM 2010 Ctrl Delay			38.3									
HCM 2010 LOS			D									

Peak Hour 9/30/16
12:30 pm to 1:30 pm

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Appendix F—Traffic Data Analysis

Lanes, Volumes, Timings (ICU)

3: English Rd & E Main St - Max 2

10/28/2016

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Volume (vph)	124	1270	108	145	1161	156	158	131	128	212	114	154
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	200		0	165		825	125		125	75		75
Storage Lanes	1		0	1		1	1		1	1		1
Taper Length (ft)	20			65			75			100		
Lane Util. Factor	1.00	0.91	0.91	*0.20	0.91	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frnt		0.988				0.850			0.850			0.850
Fit Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1787	5074	0	357	5136	1599	1787	1881	1599	1787	1881	1599
Fit Permitted	0.209			0.080			0.673			0.445		
Satd. Flow (perm)	393	5074	0	30	5136	1599	1266	1881	1599	837	1881	1599
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		11				170			170			167
Link Speed (mph)		35			35			30				30
Link Distance (ft)		949			1206			517				599
Travel Time (s)		18.5			23.5			11.8				13.6
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	1%	1%	1%	1%	1%	1%	1%	1%	1%	1%	1%	1%
Adj. Flow (vph)	135	1380	117	158	1262	170	172	142	139	230	124	167
Shared Lane Traffic (%)												
Lane Group Flow (vph)	135	1497	0	158	1262	170	172	142	139	230	124	167
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		28			28			12				12
Link Offset(ft)		0			0			0				0
Crosswalk Width(ft)		30			50			16				16
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	1	2		1	2	1	1	2	1	1	2	1
Detector Template	Left	Thru		Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Leading Detector (ft)	20	100		20	100	20	20	100	20	20	100	20
Trailing Detector (ft)	0	0		0	0	0	0	0	0	0	0	0
Detector 1 Position(ft)	0	0		0	0	0	0	0	0	0	0	0
Detector 1 Size(ft)	20	6		20	6	20	20	6	20	20	6	20
Detector 1 Type	CI+Ex	CI+Ex		CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(ft)		94			94			94				94
Detector 2 Size(ft)		6			6			6				6
Detector 2 Type		CI+Ex			CI+Ex			CI+Ex				CI+Ex
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0				0.0
Turn Type	pm+pt	NA		pm+pt	NA	Free	pm+pt	NA	Free	pm+pt	NA	Perm
Protected Phases	1	6		5	2		3	8		7	4	
Permitted Phases	6			2		Free	8		Free	4		4

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Appendix F—Traffic Data Analysis

Lanes, Volumes, Timings (ICU)
3: English Rd & E Main St - Max 2

10/28/2016

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector Phase	1	6		5	2		3	8		7	4	4
Switch Phase												
Minimum Initial (s)	5.0	10.0		5.0	10.0		5.0	5.0		5.0	5.0	5.0
Minimum Split (s)	11.0	25.0		11.0	25.0		11.0	25.0		11.0	25.0	25.0
Total Split (s)	30.0	50.0		30.0	50.0		20.0	35.0		20.0	35.0	35.0
Total Split (%)	22.2%	37.0%		22.2%	37.0%		14.8%	25.9%		14.8%	25.9%	25.9%
Maximum Green (s)	24.0	44.0		24.0	44.0		14.0	29.0		14.0	29.0	29.0
Yellow Time (s)	4.0	4.0		4.0	4.0		4.0	4.0		4.0	4.0	4.0
All-Red Time (s)	2.0	2.0		2.0	2.0		2.0	2.0		2.0	2.0	2.0
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	0.0
Total Lost Time (s)	6.0	6.0		6.0	6.0		6.0	6.0		6.0	6.0	6.0
Lead/Lag	Lead	Lag		Lead	Lag		Lead	Lag		Lead	Lag	Lag
Lead-Lag Optimize?												
Vehicle Extension (s)	1.0	1.0		1.0	1.0		1.0	1.0		1.0	1.0	1.0
Recall Mode	None	Max		None	Max		None	None		None	None	None
Walk Time (s)		4.0			4.0			4.0			4.0	4.0
Flash Dont Walk (s)		15.0			15.0			15.0			15.0	15.0
Pedestrian Calls (#/hr)		0			0			0			0	0
Act Effct Green (s)	52.6	44.1		74.1	59.6	117.7	24.0	12.3	117.7	27.1	13.9	13.9
Actuated g/C Ratio	0.45	0.37		0.63	0.51	1.00	0.20	0.10	1.00	0.23	0.12	0.12
v/c Ratio	0.49	0.79		1.86	0.49	0.11	0.56	0.72	0.09	0.77	0.56	0.50
Control Delay	18.5	36.5		451.0	20.9	0.1	42.2	72.1	0.1	54.5	59.7	12.6
Queue Delay	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	18.5	36.5		451.0	20.9	0.1	42.2	72.1	0.1	54.5	59.7	12.6
LOS	B	D		F	C	A	D	E	A	D	E	B
Approach Delay		35.1			61.4			38.7			42.3	
Approach LOS		D			E			D			D	

Intersection Summary

Area Type: Other
 Cycle Length: 135
 Actuated Cycle Length: 117.7
 Natural Cycle: 110
 Control Type: Actuated-Uncoordinated
 Maximum v/c Ratio: 1.86
 Intersection Signal Delay: 46.3
 Intersection LOS: D
 Intersection Capacity Utilization 73.6%
 ICU Level of Service D
 Analysis Period (min) 15
 * User Entered Value

Splits and Phases: 3: English Rd & E Main St



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